

# हुगली कोचीन शिपयार्ड लिमिटेड

#### HOOGHLY COCHIN SHIPYARD LIMITED

(A Govt. of India Enterprise)



वसुधेव कुटुम्बकम् ONE EARTH • ONE FAMILY • ONE FUTURE

Date: 03.10.2025

#### **CORRIGENDUM-3**

Ref. HCSL/PUR/TEN/2025/071 dt 12.08.2025

Sub: Corrigendum for Publishing Reply for Pre-Bid Queries.

**TENDER FOR** "Turn Key- Engineering, Supply, Installation, and commissioning of one number Boat Hoist/ Lift of 800T capacity for the Ship Repair Facility at Pandu, Guwahati, Assam, India".

The Pre-Bid Meeting for Tender for "Turn Key- Engineering, Supply, Installation, and commissioning of one number Boat Hoist/ Lift of 800T capacity for the Ship Repair Facility at Pandu, Guwahati, Assam, India" was conducted online on 10<sup>th</sup> September 2025.

Further to the Pre bid meeting, reply for the pre-bid queries are herewith attached as Enclosure -I.

Design Project Report is also attached herewith as Enclosure-II.

All other terms & conditions shall remain unaltered.

For Hooghly Cochin Shipyard Limited















				REPLY TO BIDDERS QUERIE'S		
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
				Stage 1- On issuance of the purchase order for the material supply. 10 % of the supply order value excluding taxes and duties against submission of equivalent bank guarantee, as per HCSL format, valid till completion of material supply and submission of bank guarantee for 10% of the Contract Price towards performance of the contract in accordance with the "Conditions of Contract" 4.2 and the "Appendix to Tender" (see Volume II).	20% advance against submission of ABG 80% against irrevocable Letter of Credit with following milestones.	Tender condition shall prevail.
				Stage 2- Delivery of boat lift materials/ items at Pandu site after certification by third party Classification society. (Prior to dispatch for items). 60% of the supply order value including all taxes and duties	60% of the supply order value including all taxes and duties after the certification of Third Party Inspection. (Prior to despatch)	
				Stage 3- Positioning of Boat lift erection equipment at the site. 10% of the supply order value including all taxes and duties and 10% of installation and commissioning value including all taxes and duties	10% of the supply order value including all taxes and duties after Completion of assembly, erection of structural frame work ,transversal beam and wheelgroups and certification by thirdparty Classification society	
1	Financial Part (Vol IV) Payment Schedule	8	09 of 16	Stage 4 - Completion of assembly, erection of structural frame work, transversal beam and wheel groups and certification by third party Classification society. 10% of the supply order value including all taxes and duties and 30% of installation and commissioning value including all taxes and duties	10%. Completion of assembly, erection of all machineries, electrical and control system components and certification by third party Classification society	
				Stage 5 - Completion of assembly, erection of all machineries, electrical and control system components and certification by third party Classification society. 10% of the supply order value including all taxes and duties and 30% of installation and commissioning value including all taxes and duties	Erection & Commissioning  1)30% - mobilization of advance before commencement of Erection & assembly of the Boat Lift parts.  2) 40% Completion of assembly, erection of all machineries, electrical and control system components and certification by third party Classification society	
				Stage 6 Completion of installation and completion of functional and operational test and certification by third party Classification society. 20% of installation and commissioning value including all taxes and duties	3) 20% Completion of installation and completion of functional and operational test and certification by third party Classification society. 4) 10% Successful commission of the boat lift, and certification by third party Classification society, providing operational training and maintenance training and receipt of BG towards performance and	
				Stage 7 Successfull commissiong of the boat lift, and certification by third party Classification society, providing operational training and maintenance training and receipt of BG towards performance and gurantee of the equipment. 10% of installation and commissioning value including all taxes and duties	guarantee of the equipment	
						Furn

REPLY TO BIDDERS QUERIE'S								
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply		
2	COC (Vol II) Delay damages	Sub Clause 8.7	14 of 26	In Sub-Clause 8.7, the sum referred to in the second sentence shall be 0.05% of the Contract Price as delay damages in respect of the Works, payable (per day) in the proportions of currencies in which the Contract Price is payable. The maximum amount of delay damages shall be ten percent (10%) of the Contract Price stated in the Contract Agreement.	Bidder request to accept the following terms In case of delay in supply and installation of complete system, liquidated damages (LD) may be levied at the rate of 1/2.% (half percent) per week or part of the week of the total basic price of undelivered portion of Equipment, subject to a maximum of 5% of the total basic price of undelivered material	Tender condition shall prevail.		
3	COC (Vol II) Time for Completion	Sub Clause 8.2	13 of 26	Time for Completion for Sections according Sub-Clause 1.1.5.6 are:  Phase 1 Preliminary Design 15 days Detailed Design 15 days Review period by the Employer 15 days Final Detailed Design 15 days Certification (design appraisal) by Third party certification agency like Lloyds Register, DNV, BV, IRS, TUV etc 15 days Phase 2 Manufacturing Phase 4 months or 120 days Delivery, Installation & Commission of Boat Lift 3 months or 90 days	We request you to accept 14 months from the date of receipt of the techno-commercially clear PO and advance payment/ Letter of Credit intimation.  Ex-works delivery: 10-11 months + 1,5 months delivery + 1,5 months for installation, commissioning, and training.	Tender condition shall prevail.  Time is the essence of the contract.		
4	ITT (Vol I) General	1.5.1 (a)	09 & 14 of 51	The Tenderers shall be a single firm. No JV or Consortium is allowed. The bidder/firm shall submit a Notary vetted Certificate of Incorporation of the company.	Please confirm whether the foreign OEM can bid directly as the prime bidder and engage a single Indian subcontractor to provide installation, commissioning support, after-sales/service, spare parts supply, and repair and maintenance works in India.  Please confirm that engaging such an Indian subcontractor will not be treated as a JV/consortium.	Refer the Tender Conditions & Defentions.		
5	ITT (Vol I) General	2.6 & 4.2	17 & 22 of 51	Tenderers shall deposit an amount of Rs 36 Lakhs (for Indian firms) / USD 41 Thousand/ EUR 35.30 Thousand only. (In case of foreign suppliers) in any hard and free convertible currency as Earnest Money Deposit (EMD) along with the tender.	Please specify the EMD amount, mode of submission, and applicability for foreign OEM bidders.  Please confirm the acceptable currency for EMD (e.g., EUR) and whether a SWIFT guarantee from an Italian bank is acceptable.	HSCL already issued response under Corrigendum-I dated 22/08/2025. Please Refer Tender Clause 2.6 for EMD.		



				REPLY TO BIDDERS QUERIE'S		
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
					Please specify tax duties and withholding requirements applicable to foreign OEM, including: (i) GST on goods; (ii) GST on services (reverse charge); (iii) Customs duty on imported goods; (iv) TDS on payments.	The bidders shall ascertain the prevailing duties and taxes on their own and submit the bid accordingly.
6	Financial Part (Vol IV) Payment Schedule		11 & 12 of 16			
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Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
				The Tenderer shall have over the past Ten (10) years experiences as original equipment manufacturer and supplier of a minimum of two (02) similar maritime Boat lift system, which have been in operation satisfactorily for at least one (01) year. The two Boat lifts systems may be part of different projects. Similar capacity shall mean Boat lift system between 400 Ton to 800 Ton with self-driven wheels and power unit. The projects shall include successful Design, Fabrication, Installation and Commissioning. Employer's certificates, third party Test Certificate	Please clarify documentation requirements for foreign OEM credentials.  Please confirm whether authentication by Indian Embassy is required for technical experience and completion certificates and specify the process.  Please confirm whether apostille under the Hague Convention is an acceptable alternative if embassy authentication is not possible.	Tender condition shall prevail.
	*		09 & 10 of 51	and copy of the Contract comprising the maximum lifting capacity of the system have to be submitted in the Tender. If documents not mentioning the Employers or the tenderers company name additionally a clarification letter is required. The reference projects have to be filled in the form according Annex 5 and information related to present projects according Annex 6. In case the work experience is for the work executed outside India, the bidders have to submit the completion/experience certificate issued by the owner duly signed & stamped,		
7	ITT (Vol I) General	1.5.2 (a)		and affidavit to the correctness of the completion/experience certificates. The Contractor shall also get the completion/experience certificates attested by the Indian Embassy/Consulate/High Commission in the respective country. In the event of submission of completion/experience certificate by the Bidder in a language other than English, the English translation of the same shall be duly authenticated by Chamber of Commerce of the respective country and attested by the Indian Embassy/consulate/High Commission in the respective country. For the purpose of evaluation of Bidders, the conversion rate of such a currency into INR shall be the daily representative exchange rate published by the SBI TT selling rate as on the Date of completion of work. HCSL reserves the right to obtain clarifications or confirmations from the clients listed by the Tenderer in their previous experience record. HCSL reserves the right to visit the boat lift installations done by the Tenderer in order to have firsthand information regarding the performance of the boat lift/hoist and after sales support provided by the supplier. The Tenderer shall co-ordinate for obtaining approval/ consent for the above-mentioned visit whereas travelling and all other expenses for the same shall be borne by HCSL. Final acceptance of the offer is subject to the receipt of satisfactory feedback from the Clients as mentioned above.		

				REPLY TO BIDDERS QUERIE'S		
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
8	Employers requirement (Vol III)	3	03 of 61	Train o mines outprij oom to	Please specify required utility capacity (e.g., kVA for power; m³/h for water).	Tender condition shall prevail. The employer, if requested by the successful bidders, shall provide the recommendatary asisstance, to enable them to approach the local authorities to avail the required utility services. However the responsibility for arranging the utility services shall always rest with the successful bidders.
9	ITT (Vol I) General	1.14	14 of 51	For the following specialized works, the Employer permits specialized Subcontractors, if required by the main contractor:  i. design and manufacturing of Boat lift control system  ii. fabrication of the boat lift structure (only steel works)  Tenderers planning to subcontract specialized works shall specify, in the Tender submission  form, the activity(ies) or parts of the works to be subcontracted along with complete details of  the Subcontractors and their qualification and experience. The Subcontractors proposed shall  be fully qualified for their work proposed, and meet the specified criteria in  Clause 1.5.1 b) - e)  & g) and Clause 1.5.2 a) for his proposed works, failing which such  Subcontractors will not be  permitted to participate. The qualification and experience of Subcontractors proposed by the  Tenderer will not be considered for evaluation of the Tenderer. The Tenderers on its own (without taking into account the qualification and experience of the Subcontractor) have to meet the post-qualification criteria. Each Subcontractor has to be listed in the form of Annex 8.	Please clarify:  (a) whether subcontracting is strictly limited to these two activities or if additional specialized works could be proposed and approved;  (b) the exact format and level of detail required in Annex 8 for listing subcontractors, including any supporting documentation for their qualifications and experience (draft Annex 8 sheets for prospective subcontractors are attached for comment on format);  (c) how the specified criteria (e.g., technical and financial thresholds) will be applied and verified for subcontractors, and whether partial compliance or equivalents from international standards (e.g., EU certifications) would be acceptable;  (d) if multiple subcontractors can be proposed for a single activity, and the process for selecting or approving them;  (e) the implications if a proposed subcontractor is rejected (e.g., whether it disqualifies the entire bid or allows substitution);  (f) confirmation that the tenderer's post-qualification evaluation relies solely on its own merits, without any aggregation of subcontractor experience or resources;  (g) confirmation that any substitute subcontractor can be proposed within seven days of HCSL rejection.	required supporting documents, as deemed appropriate.  (c) Tender condition alone shall prevail. The successful bidders are expected to follow the tender stipulations while nominating the sub contractors.  (d) It is the responsibilty of the successful bidders to select the suitable sub contractors who has sufficent previous experience and expertise.  (e),(f)&(g) Tender condition shall Prevail.



				REPLY TO BIDDERS QUERIE'S		
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
					particularly in the context of customs duties under Indian Customs Law (Chapter 89) for ship repair facilities.Please clarify: (a) whether the consignee is Inland Waterways or HCSL; (b) if Inland Waterways is designated as the consignee (as the actual user) and their PANDU facility is registered as a Ship Repair Facility (SRF) under the Directorate General of Shipping, whether the OEM's export can be made directly in the name of Inland Waterways instead	Tender condition shall prevail. The bidders shall ascertain the applicable duties and taxes, customs duties, fees, royalities, etc., and submit a single rate proposal as specifie in the bid document.
10	ITT (Vol I) General	2.8.2.5	18 of 51		of HCSL; (c) confirmation that, if export is allowed to Inland Waterways as the actual-user consignee, the OEM would be eligible to waive customs duty calculations under Chapter 89 of the Indian Customs Tariff Act, including any specific conditions or documentation required for such exemption; (d) the consignee's GSTIN / IEC that must appear on shipping documents.	
11	ITT (Vol I) General	2.8.2.6	18 & 19 of 51	For Overseas firms Applicable taxes in India (Income tax and GST extra as applicable) shall be borne by the Supplier (As per Income tax act,1961 for Indian suppliers and Income Tax Act, 1961 and DTAA agreements in the case of foreign vendors). Income tax liability of non-resident service engineer based on his period of stay in India will not be borne by HCSL. In all cases, the non-resident vendor/service provider shall provide such documents that are necessitated by the Indian income tax laws so as to enable HCSL to comply with the provisions of Indian statute and for payments of income tax in India. Following documents shall be sought by HCSL in this regard (i) Certificate under 10 (F) (ii) Tax residency certificate (iii) The certification regarding the existence/non-existence of business connection or permanent establishment in India (The above is only an indicative list) Note - Filing of Form 10F is to be done on line which is mandatorily required to be complied with for availing DTAA benefit If overseas bidder is not agreeing for above clause, loading factor will be considered during price comparison for arriving L1. For Indian firms- Income tax will be deducted from payments. GST shall be paid extra as applicable.	the India-Italy DTAA, including withholding tax (TDS) rates and processes for availing DTAA benefits (e.g., reduced rates for business profits, royalties, or technical services, with required documentation like Tax Residency Certificate or Form 10F); (b) criteria for determining "income accruing or arising in India," including Permanent Establishment (PE) thresholds (e.g., based on transaction value, user interactions, or physical presence) and its impact on taxation; (c) whether HCSL will withhold taxes at source and the mechanism for claiming relief; (d) GST handling for imports, including applicability of reverse charge, registration requirements for foreign suppliers without	The bidders shall ascertain the prevailing duties, taxes etc., on their own and submit to bid accordingly.  Refer tender Condition; VOLUME IV Financial Part, Clause 8. Payment Schedule

REPLY TO BIDDERS QUERIE'S									
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply			
					We intend to subcontract to an Indian company all the works and activities to be performed on site (i.e. site preparation, erection, commissioning, load test).	Tender condition shall prevail.			
12	ITT (Vol I) General	1.14	37 of 51						
-	(1017) 00.00.00								
					We kindly ask to accept the following payment conditions: FOR SUPPLY (included taxes and duties) Stage 1 Advance Payment - On issuance of the purchase order for the Boat Lift: 20 % of the contract order FOR THE SUPPLY against submission of equivalent bank guarantee, valid for 10% at FAT certificate and 10% till the commissioning date. Stage 2 - 80% by means of an irrevocable and confirmable L/C to be issued via a first-rate international bank not later than one month from the execution of this contract agreement and payable as follows:  •30% of the contract value FOR THE SUPPLY after the FAT test at				
					Seller's premises duly signed from a third-party inspection  •30% of the contract value FOR THE SUPPLY against shipping documents .  •10% of the contract value FOR THE SUPPLY upon arrival on site in Pandu.  •10% of the contract value FOR THE SUPPLY after assembly, erection and commissioning in Pandu.  FOR				
13	Financial Part (Vol IV) Payment Schedule	Para 8	9 of 16		ERECTION AND COMMISSIONING (included taxes and duties)  1.30% - mobilization of advance before commencement of Erection  & assembly of the Boat Lift parts.	इन्तामरे			

				REPLY TO BIDDERS QUERIE	S	
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
					2.40% Completion of assembly, erection of all machineries, electrical and control system components and certification by third party Classification society 3.20% Completion of Installation and completion of functional and operational test and certification by third party Classification society. 4.10% Successful commission of the boat lift, and certification by third party Classification society, providing operational training and maintenance training and receipt of BG towards performance and guarantee of the equipment	
					Please clarify the following sentence: Towards stoge 1 payment, a Bank Guarantee In HCSL format equivalent to the stage 7 payment, to cover the period till entire completion of material supplies is to be furnished. On receipt of bank guarantee and receipt of performance security in accordance with the "Conditions of Contract" 4.2 and the "Appendix to Tender" (see Volume II), stage I payment shall be released.	
14	Financial Part (Vol IV) Payment Schedule	Para 9	10 of 16			
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	REPLY TO BIDDERS QUERIE'S								
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply			
					OEM estimates the following time of completion: Phase I Preliminary Design: 15 days Detailed Design: 30 days Review period by the Employer: 15days Final Detailed Design: 15 days Certification (design appraisal) by a third-party certification agency like Lloyds Register, DNV, BV, IRS, TUV etc: 15 days Phase 2 Manufacturing Phase: 8-9 months Delivery, Installation Commission of machine: 4-5 months				
15	COC (Vol II) Time for Completion	Sub Clause 8.2	13 of 26						
						Salimi J Dielo			

				REPLY TO BIDDERS QUERIE'S		
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
			2 2	the procurement document should specify.	Not acceptable. The majority of the components and materials for our Boat Hoist are standards and already stocked in our warehouse. These materials and components have been selected over the years, optimized and adapted for our machines. A list of main components will be included in our technical specification, and we'll also include the drawings and specs. of the main ones.	
16	Employers requirement (Vol III) Material control		26 of 61			
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#### TENDER NO: HCSL/PUR/TEN/2025/071 dated 12.08.2025

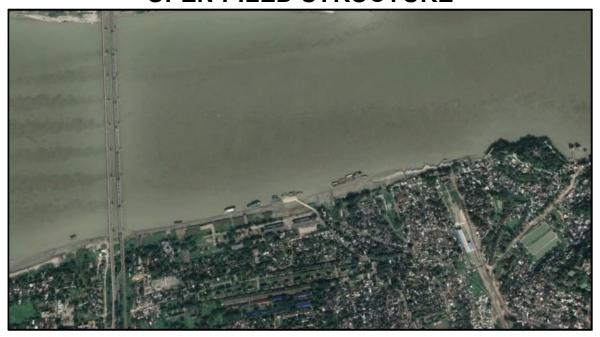
# REPLY TO BIDDERS QUERIE'S Reference **HCSL Reply** Sl.No **Query Reference** Page No Description **Bidder's Query** Clause Relationship with Civil works design In the document, it is stated that 'The boat hoist Contractor shall interact with the Consultant as required to collect information about the civil design loads and civil works executed, and the Contractor should take that information into consideration while designing the boat lift system." We would like to clarify that the ground reactions and dimensions of the Boat Hoist will be included in the tender documentation (preliminary drawing) and will also be confirmed during the final design phase. The Contractor is not able to take on any additional activities, checks, or verifications related to the civil Employers 17 22 of 61 requirement (Vol III)

REPLY TO BIDDERS QUERIE'S								
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply		
18	COC (Vol II) Delay damages	Sub Clause 8.7	14 of 26		In Sub-Clause 8. 7, the sum referred to in the second sentence shall be 0.05% of the Contract Price as delay damages in respect of the Works, payable (per day) In the proportions of currencies in which the Contract Price is payable. The maximum amount of delay damages shall be ten percent (10%) of the Contract Price stated in the Contract Agreement. We propose the following:  In Sub-Clause 8.7, the sum referred to in the second sentence shall be 0.05% of the Contract Price as delay damages in respect of the Works, payable (per day) in the proportions of currencies in which the Contract Price Is payable. The maximum amount of delay damages shall be five percent (5%) of the Contract Price stated in the Contract Agreement.			
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				REPLY TO BIDDERS QUERIE'S		
Sl.No	Query Reference	Reference Clause	Page No	Description	Bidder's Query	HCSL Reply
					OEM propose the following:  2: B = A+ 90 days  3: C = B + 30 days **  4: D = C + 240/270 days  E = D + 120 /150 days  5:	
19	Employers requirement (Vol III) Key milestone dates		57 of 61			
20	Corrigendum I dated 22.08.2025	Clause 2.12			Due to the limited time available between the Pre- Tender Meeting (10th September 2025) and the deadline for receiving the tender documents (19th September 2025), and considering the long shipping times for foreign bidders as well as the potential amendments arising from the Pre-Tender Meeting and the consequent adjustments required in the tender documents, we kindly request the possibility to extend the submission deadline to the first week of October.	HSCL already issued response under Corrigendum-2 dated 17/09/2025.
21	Corrigendum I dated 22.08.2026		L	pad tests	Since we are facing difficulties in sourcing the 1000 tons of loads required to perform the tests (static & dynamic), we kindly request whether it would be possible to exclude from the scope of supply the part related to the load tests and, alternatively, to receive support from the Employer who, being a local shipyard, may have easier access to such loads such as a suitable boat/barge or other types of heavy loads.	The request cannot be accepted. Further the employer cannot guarantee extension of the requested support at aplace of testing. It is the responsibility of the successful bidder to scout a suitable mechanism to perform the intendent test.

# DESIGN PROJECT REPORT FOR SHIP REPAIR FACILITY TO HANDLE INLAND WATERWAYS VESSELS AT PANDU, GUWAHATI

# **OPEN PILED STRUCTURE**



Client Hooghly Cochin Shipyard Limited.

Consultants
Prof. S.A.Sannasiraj
Prof. K. Murali





National Technology Centre for Ports, Waterways and Coasts

Department of Ocean Engineering Indian Institute of Technology Madras Chennai – 600 036

Client	Cochin Shipyard Limited	Date	26/03/2022
Project	Design Project Report for Ship Repair Facility to handle Inland waterways Vessels at Pandu, Guwahati	Consultant	IITM/ NTCPWC



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	Design Project Report for Ship Repair Facility to handle Inland waterways Vessels at Pandu, Guwahati	Consultant	IITM/ NTCPWC



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#### 1.0 Introduction

Pandu Port is a riverine port in the Indian state of Assam, serving North Eastern states of Assam and Guwahati in particular. This port has been developed on the bank of the Brahmaputra River. The port is strategically important and one of the largest river port in the north eastern state of the country wherein, in order to accommodate ship anchorage, a naval terminal or permanent jetty is existing. In addition, many passenger ships are anchored with the tourists at this port. Considering the huge demand for repair of the vessels plying in this route and also the additional cost and time consuming before reaching the other ship repair yards nearby, the state government has embarked an ambitious plan to develop the port with emphasis on development of ship repair facilities and also allocated funds for the modernization of the port under the National Waterways development projects. The Port is also included in the 'Port of call' agreement between India and Bangladesh.

The ship repair facility is proposed on the eastern side of existing jetty at Pandu Port at a distance of about 350m. The latitude and longitude of the ship repair facility is 26°10'15.01"N and 91°40'59.87"E. The aerial view of Pandu port and the location of ship repair is shown in **Fig. 1.1** and **Fig. 1.2**, respectively.

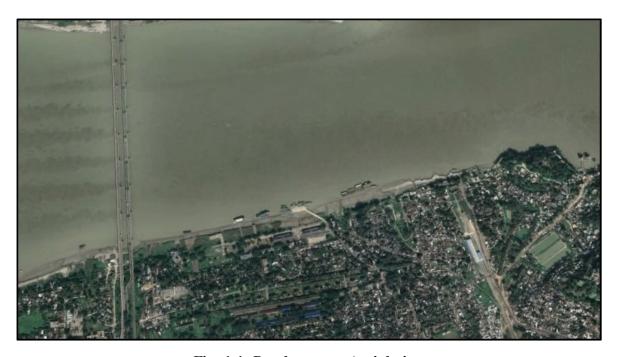


Fig. 1.1: Pandu port – Aerial view

Client	Cochin Shipyard Limited	Date	26/03/2022	TUTE
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Fig. 1.2: Location of ship repair facility

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#### 2.0 Scope and Objective of the Project

- a) Visit the proposed site and collect the data available with IWAI/HCSL and other authorities, required for the design of Ship repair facility.
  - b) Identification of additional investigation / collection of data, if any required.
- c) HCSL will directly carry out Soil Investigation and Topographical Survey. Consultant has to do supervision of this Soil Investigation and Topographical Survey. Consultant has to also immediately submit to HCSL the detailed scope of soil investigation/topographical survey with layout of boreholes along with detailed specification for soil investigation and Topographical Survey.
- d) Carry out Structural stability study of the existing foundations I structures, buildings, etc. which are intended for further use, as per the tentative master plan. Submit recommendations for refurbishment/ restoration of above facilities, if the present structural condition of these are not complying with currently acceptable standards.
  - e) Preparation of an Inception Report.
  - f) Review existing layout plan of the ship repair facility.
  - g) Preparation of final layout Plan/ land use plan.
  - h) Preparation of Basis of design.
  - i) Preparation of Detailed Project Report.
  - i) Taking approvals for statutory authorities for the project. Design & Detailed Engineering.
  - k) Preparation of EIA report and providing Consultancy for obtaining.
  - 1) Environmental Clearance from appropriate authorities.
  - m) Preparation of Detailed cost estimate.
  - n) Preparation of tender drawings (for each packages like civil, electrical & mechanical).
- o) Preparation of Tender documents along with Pre-qualification of contractors. (For each packages like civil, electrical & mechanical).
- p) Evaluation of Technical and Financial Bids. (For each packages like civil, electrical & mechanical).
- q) Preparation of Work Order / Purchase Order and all necessary agreements with the Contractors. (For each packages like civil, electrical& mechanical).
  - r) Preparation and issuance of Good for Construction drawings.
  - s) Site Project Management and Supervision during construction & commissioning.

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- t) Review, Comment and Approval of drawings and design calculations, if any submitted by the contractors.
  - u) Render professional advice as and when required.
  - v) Prepare and submit as-built drawings at the end of construction.
  - w) Inspection during Defect Liability Period of Contractor.
  - x) Assist HCSL on Audit queries.
- y) Responsibility on technical details, overall performance of the system and timely completion of project.

The present report covers the design project report, cost estimation and drawings for ship repair facility to handle the inland water ways vessels.

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#### 3.0 Revisions and standards

# 3.1 Revision History

# **Drawings:**

S. No.	Report No	Description	Revision	<b>Issued On</b>	Remarks
1	IITM-PU-SR-001	Issued	A		

# Report:

S. N	o Report No	Description	Revision	<b>Issued On</b>	Remarks
1	IITM-PU-RPT-001	Issued	A		

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### 3.2 Changes to this Revision

The structural cross-section of the ship repair facility is presented in Fig. 1.3.

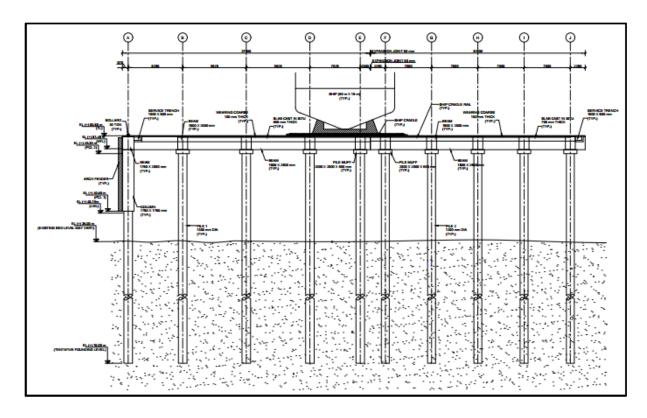


Fig. 1.3: Cross-section of ship repair facility

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#### 3.3 Codes and Standards

The codes and standards for the design of the ship repair facility structure are given below.

Code	Description			
IS: 875 Part 3	Code of Practice for Design Loads for Building and Structures			
15: 6/5 Fart 5	Wind Load			
IS: 456	Code of Practice for Plain and reinforced Concrete			
IS: 1893 Part 1	Criteria for Earthquake-resistant Design of structures			
IRC 6 – 2014	Standard specifications and code of practice for roads, bridges,			
IKC 0 - 2014	section II Loads and stresses.			
IS: 4651 Part 1	Site Investigation.			
IS: 4651 Part 2	Earth Pressure.			
IS: 4651 Part 3	Loadings			
IS: 4651 Part 4	Design Considerations			
IS: 4651 Part 5	Layout and Functional requirements			
IS 2911-2010	Design and construction of Pile Foundations – Concrete Piles.			
Part 1 / Sec 2	Design and construction of the Foundations – concrete thes.			
SP16	Design aids to IS: 456			
SP34	Handbook on Concrete Reinforcement and Detailing			
IS 800	General Construction in Steel			

Note: Unless noted otherwise, the latest edition of the above codes & standards shall be used.

#### 3.4 Reference

- [1] Foundation Analysis and Design by Joseph E. Bowels- 5th edition McGraw Hill book.
- [2] Safe Berthing and Mooring Trelleborg Marine Systems.
- [3] Limit State design of reinforced concrete by P.C. Varghese.

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#### 4.0 Outline of the Report

The report has been organized as per the following sequence.

Section 1: Introduction.

Section 2: Scope and Objective of the Project.

Section 3: Revisions and Standards.

Section 4: Outline of the Report.

Section 5: Project Site Environment.

Section 6: Design Information's for Ship repair facility structures.

Section 7: General Arrangement and Structural Model for Proposed Structure.

Section 8: Loadings for Ship repair facility structures .

Section 9: Structural Analysis Results.

Section 10: Results and Discussions.

Section 11: Summary.

1				
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#### **5.0 Project Site Environment**

The analysis of the prevailing environmental aspects of the project site is essential for the better understanding of the site that will help in development and modification of the ship repair facility. Hence an overall environmental data on the location, connectivity, meteorological parameters, geotechnical aspects, and basic details of the project site are reproduced from the earlier reports for better appreciation.

#### 5.1 Water level

The maximum and minimum water levels observed at Pandu over a period of 2004-2014 are tabulated in **Table. 5.1**. The observed Highest Flood level at Pandu is 50.05 m and lowest water level at Pandu is 40.31 m with reference to the mean sea level.

Table. 5.1: Maximum and Minimum water levels observed at Pandu

		Year								
		2004	2005	2006	2007	2008	2009	2010	2011	2012
Tannar	Max.	41.43	41.56	41.04	40.94	41.74	-	41.16	41.52	40.89
January	Min.	41.26	41.20	40.50	40.41	41.38	-	40.52	41.10	40.38
Fohmom	Max.	41.26	42.33	41.11	40.79	41.78	-	40.50	41.45	40.70
February	Min.	41.11	41.06	40.31	40.39	41.43	-	40.35	40.99	40.52
Mayah	Max.	44.64	44.28	42.21	40.98	42.57	-	43.43	43.66	41.34
March	Min.	41.06	41.75	40.92	40.51	41.43	-	40.40	40.94	40.60
Amril	Max.	45.39	45.15	-	-	42.92	-	47.26	44.13	44.24
April	Min.	42.48	43.21	-	-	42.92	-	43.13	42.28	41.31
May	Max.	47.09	44.91	44.91	47.18	-	44.51	48.05	44.98	46.13
May	Min.	43.03	43.60	44.01	44.57	ı	42.61	44.16	42.94	43.19
June	Max.	48.21	46.84	48.70	48.34	-	45.29	48.79	46.27	48.87
June	Min.	44.85	44.46	46.20	46.09	1	43.64	46.21	44.17	44.59
Turky	Max.	49.20	47.73	48.42	50.05	-	47.42	48.91	47.84	48.47
July	Min.	46.88	46.02	47.24	47.16	-	45.40	48.30	45.71	47.22
August	Max.	47.56	47.89	47.45	50.05	-	47.78	49.39	47.79	48.16
August	Min.	46.73	46.02	45.57	47.63	-	46.53	47.41	46.22	46.35
Contombox	Max.	47.54	47.41	48.59	48.13	-	46.75	49.92	46.88	49.40
September	Min.	45.42	44.38	45.87	46.17	-	45.05	48.09	45.21	46.18
October	Max.	47.64	45.39	46.15	45.92	-	46.22	-	46.58	48.23
October	Min.	43.65	43.44	45.42	44.40	-	42.86	-	42.87	44.17
November	Max.	43.82	43.76	-	44.19	-	43.36	-	42.81	-
November	Min.	42.12	42.00	1	42.62	-	42.45	-	41.65	-
December	Max.	42.15	41.96	-	42.56	-	41.90	-	41.63	-
December	Min.	41.57	41.07	-	41.76	-	41.19	-	40.93	-

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It could be observed that during the months of December to March the water levels are lowest, while during the months of April to October these are highest.

#### 5.2 Rainfall

Annual rainfall in the catchment area varies widely from over 250 cm in the Assam Valley to less than 50 cm in Tibet. Rainfall in the region is negligible from November to March. The rainfall commences from April and reaches its peak towards end of May. The average annual rainfall is about 160 cm at Pandu.

#### 5.3 Current

From available data on river velocities, the mean velocity during the normal and flood periods has been observed to be about 1 m/s and 4 m/s respectively. The annual records of maximum and minimum mean velocities of the river Brahmaputra at Pandu are tabulated in **Table. 5.2.** It could be observed that during months of higher discharge viz. June to September the maximum velocities could reach 3.0 to 5.0 m/s. These velocities are too high for the vessel to align along the slipway. However during these months minimum velocities are well below 2.0 m/s which could be considered as limit upto which the vessels could be handled at the slipway safely. The ship/vessel would be placed in position using tug boats of adequate capacity during monsoon season so as to transfer it to trolley for hauling upto slipway. Additional mooring structure such as buoy shall be installed during slipway operation.

Table. 5.2: Maximum & Minimum Mean Velocities (m/s) of the River Brahmaputra at Pandu

Months		1959	1961	1977	1978
Jan.	Max.	0.52	0.53	1.16	0.83
	Min.	0.39	0.47	0.83	0.73
Feb.	Max	0.54	0.53	1.10	0.86
	Min.	0.34	0.47	0.76	0.75
Mar.	Max	0.58	0.80	1.31	0.93
	Min.	0.38	0.47	0.89	0.72
Apr.	Max.	1.19	1.35	1.49	1.28
	Min.	0.47	0.68	1.02	0.86
May	Max.	2.21	2.74	2.05	1.76
	Min.	0.76	0.95	1.05	1.12
June	Max.	3.14	2.50	2.61	3.63
	Min.	1.28	1.52	1.76	1.29
Jul.	Max.	2.14	3.21	3.00	2.57
	Min.	1.55	1.50	2.00	1.70
Aug.	Max.	5.57	5.85	3.11	2.58

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	Min.	1.61	1.84	2.55	1.63
Sept.	Max.	2.10	2.19	2.90	2.16
	Min.	1.36	1.24	1.65	1.60
Oct.	Max.	1.50	1.53	2.40	1.84
	Min.	1.08	0.92	1.16	1.20
Nov.	Max.	1.15	0.97	1.14	1.33
	Min.	0.64	0.57	0.95	0.97
Dec.	Max.	0.66	0.64	1.12	1.02
	Min.	0.32	0.45	0.82	0.78

#### **5.4** Temperature

Guwahati enjoys moderate weather with average high temperature of 29°C and average minimum is 19°C. Summer (April-May) has a maximum of 40°C and winter (October to March) has a minimum 5°C.

#### **5.5 Geotechnical Data**

The geotechnical investigations at the proposed shiprepair location were carried and considered marine borehole details is given in **Table. 5.3** below.

Table. 5.3: Details of Borehole

SOIL TE	ST BO	RE H	OLE	CH	AR7		
RELIANT	ENGINEER	S, SIX-MILE	, GUW	AHATI-2	2		
BORE HOLE NO .: MAH -1 TYPE OF BORING: ALGAR & D. il NAME OF PROJECT: POYF. LOCATION: ROLLING	eig			ATE OF	COMPL	ETED:	1/202
R.L. OF BH: SOIL STRATA	Depth	Sample	type		S.P.T.		N vale
	(M)	No.		15cm	15cm	15cm	11.000
Grayish Silty ely from 15to	1.2			1	1	2	3
3.0 M	.9.0			2	2	3	5
Grayil Etty alay dom 1500	4.2			3	11	14	25
the from the seld	6.0	- V		11	12	21	39
Grayish fine sand from	7.5			12	19	24	43
6.0 m uple 9.00	9.0			13	24	27	51
Grayish Medium to fine land	10.7			15	24	29	56
WO. EJ CH WOL WILL	12.€			16	26	35	61
-do-	13.2			12	23	36	64
	15.0			20	31	32	69
-do-	16.5			22	33	AI	Th
Gravish Brown colone	19.0			24	35	45	20
mediation to fine Sanot.	19:5			23	37	43	SC
brom 18.00 on to 35.00 m.	21.0		1	27	33	42	57
	22.5			30	38	47	85
Grayish Brown colour	24.0			23	36	99	85
35:00 m to 39:00 m	2.5.5		-	2:6	38	97	85
22.00 .10 .00 32.00 40	23.0			31	42	48	00
	25.5 .			23	44	46	90
	30.0			30	41)	45	9 3
	31 · P			29	47	40	96
	33.0			31	40		93+
	34.5			34	45	50 +	98+
	36.0				-	100	- 0.
	37.50						-
	33.0				-		-
	40.50						

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#### 5.6 Topographic Information

The topographical survey of the proposed slipway at Pandu was carried out and the levels of existing surface are found to vary from 40.0 m to 52.0 m with respect to mean sea level. The topographic details of Project site as shown in **Fig. 5.1**.

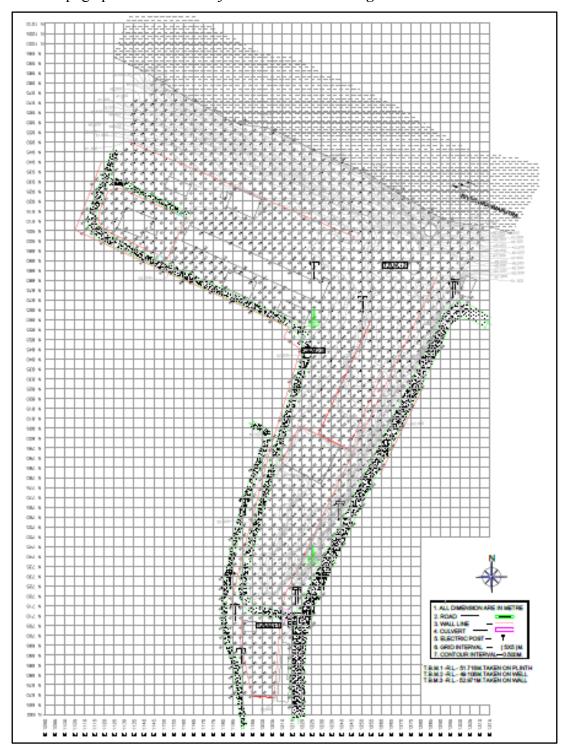


Fig. 5.1: Topography details of site

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#### 5.7 Hydrographic Information

The hydrographic survey of river Brahmaputra near the slipway location was collected by IWAI and details are shown in **Figure. 5.2**.

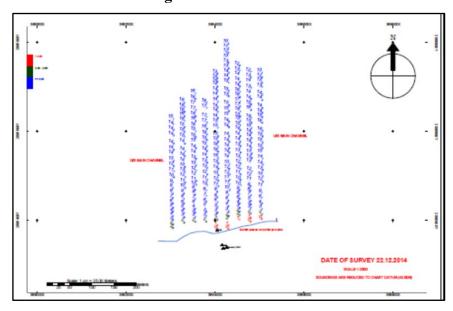


Fig. 5.2: Details of Hydrographic Survey near shiprepair facility

#### 5.8 Seismicity

Pandu is in Zone V of Indian Map of Seismic zones (IS-1893 Part-1 2016) which is a very severe risk seismic intensity zone (**Fig.5.3**).

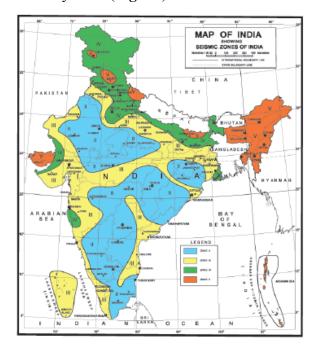


Fig. 5.3: Seismic Zoning Map of India as per IS-1893 Part 1-2016

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#### 5.9 Layout

The proposed layout consists two phase of development and it is shown in **Fig. 5.4** and **Fig. 5.5**.

The phase-I development consist of open piled structure of ship repair facility having length of 128m and width of 70m. The facility consist of boat hoist jetty of 60m length and width of 10m parallel to the river and the transfer bay consist of berthing jetty provisions along the river side. The transfer bay has length of 68m and width of 37.5m. The repair yard length is 68m and width is 32.5m. In phase-I, 60m length of vessesls can be handled in ship repair facility. The light weight of ship is approximately 500T for 60m vessels.

The phase-II development consist of open piled structure of ship repair facility having length of 217m and width of 133m. The facility consist of boat hoist jetty of 90m length and width of 10m parallel to the river and the transfer bay consist of berthing jetty provisions along the river side. The transfer bay has length of 116 m and width of 37.5m. The repair yard length is 116m and width is 95.5m. Further, the existing wet basin shall be converted as ship repair yard in Phase-II development to handle the higher size of vessels. In phase-II, 90m length of vessesls can be handled in ship repair facility. The light weight of ship is approximately 800T for 90m vessels. Accordingly, the crane dimesions shall be customised to lift the vessels.

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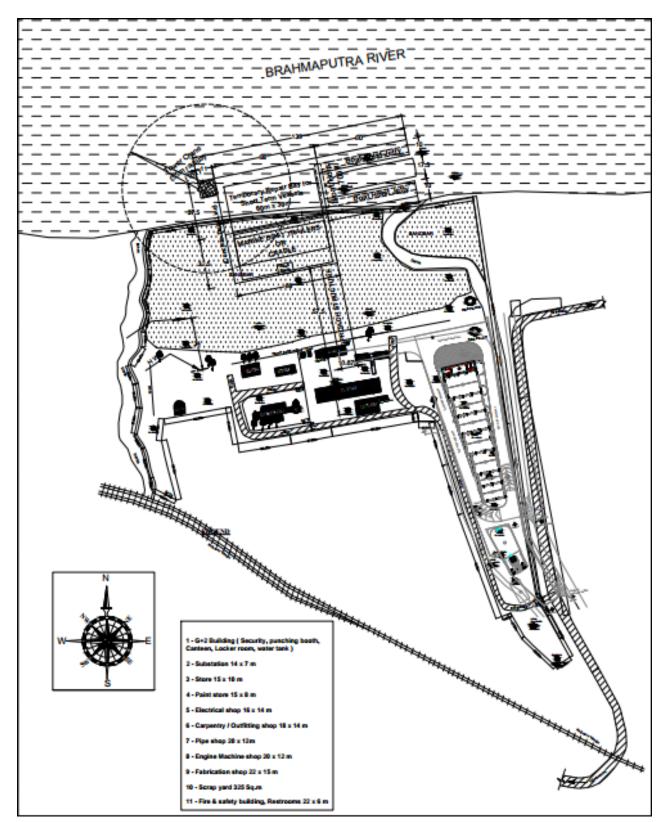


Fig. 5.4: Development of Phase –I layout

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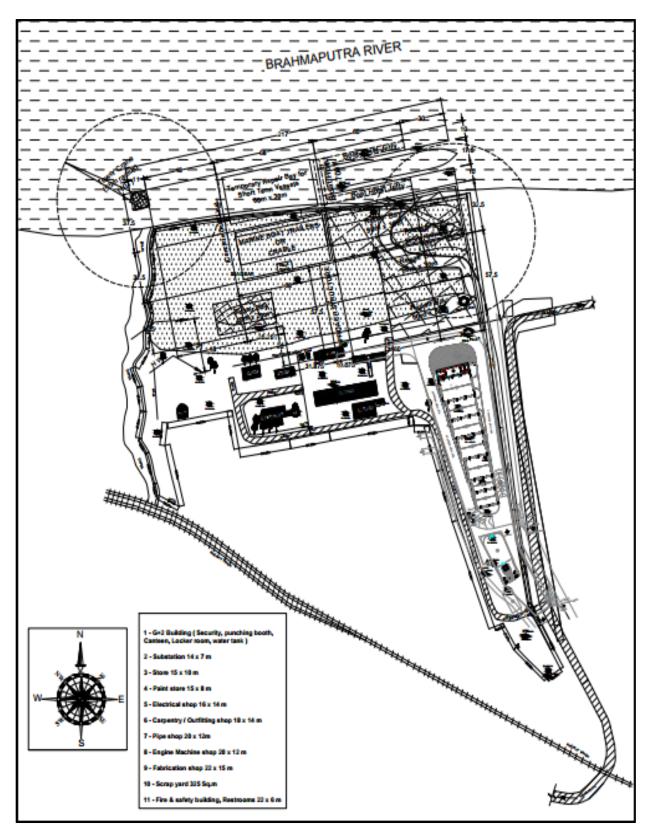


Fig. 5.5: Development of Phase –II layout

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#### 6.0 Design Information's for Ship Repair Facility

#### 6.1 General

The ship repair facility consist of boat hoist jetty, transfer bay and repair bay. The proposed cross section of boat hoist jetty consists of open piled type of jetty and the pile diameter is 1.3m. The length of jetty is 60m and width is 10m. The transvers and longitudinal beam dimension is 1.5m x 2.0m and 1.75m x 2m. The adopted thickness of deck slab is 700mm and the wearing coat of 100mm is provided. The deck level of jetty is +53.50m (CD). The plan view and cross-section of the typical jetty is shown in **Fig. 6.1** and **6.2** respectively.

The proposed cross section of transfer bay consists of open piled type of jetty and the pile diameter is 1.3m. The length of jetty is 68m and width is 37.5m. The transvers and longitudinal beam dimension is 1.5m x 2.5m and 1.75m x 2m. The adopted thickness of deck slab is 800mm and the wearing coat of 100mm is provided. The deck level of jetty is +53.50m (CD). The plan view and cross-section of the typical jetty is shown in **Fig. 6.3** and **6.4** respectively.

The proposed cross section of repair bay consists of open piled type of jetty and the pile diameter is 1.2m. The length of jetty is 68m and width is 32.5m. The transvers and longitudinal beam dimension is 1.5m x 2m. The adopted thickness of deck slab is 700mm and the wearing coat of 100mm is provided. The deck level of jetty is +53.5m (CD). The plan view and cross-section of the typical jetty is shown in **Fig. 6.5** and **6.6** respectively. The general arrangements of ship repair facility structure is attached in **Annexure-I**.

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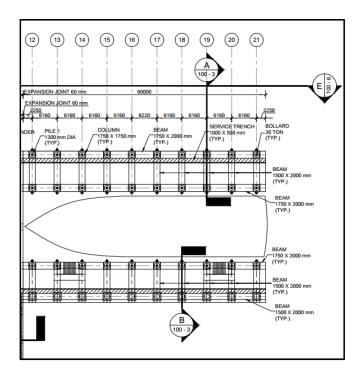


Fig. 6.1: Plan view of boat hoist jetty

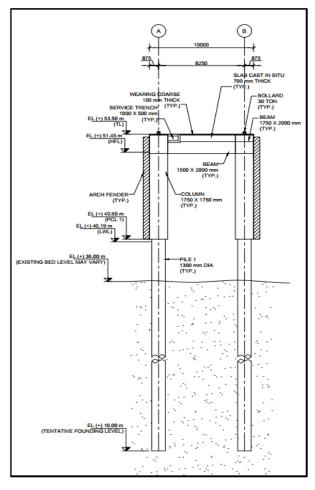


Fig. 6.2: Cross-section of boat hoist jetty

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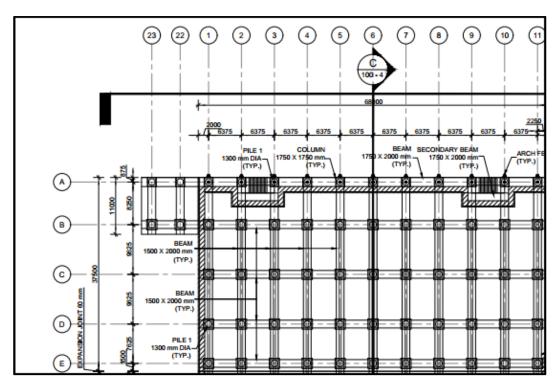


Fig. 6.3: Plan view of the transfer bay

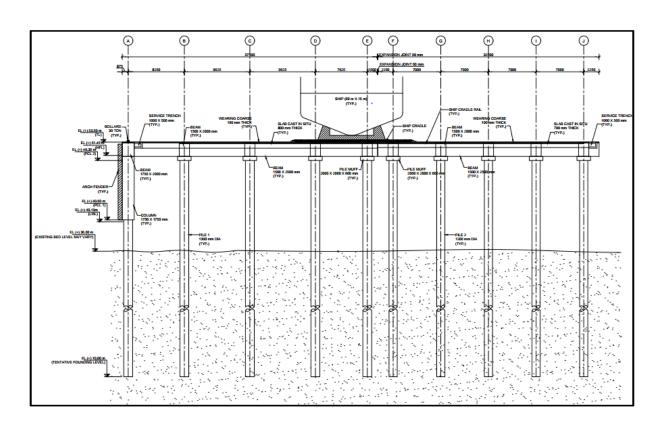


Fig. 6.4: Cross-section of transfer bay

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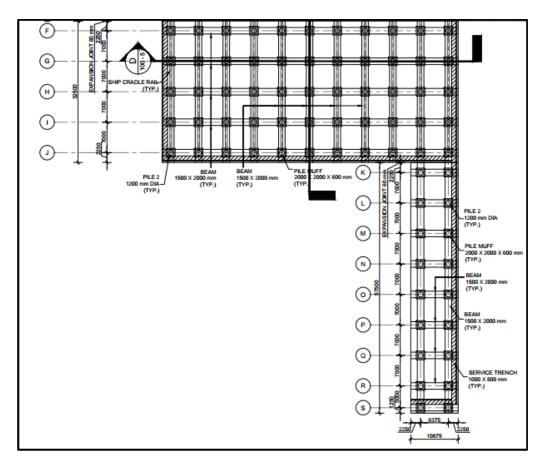


Fig. 6.5: Plan view of the ship repair bay

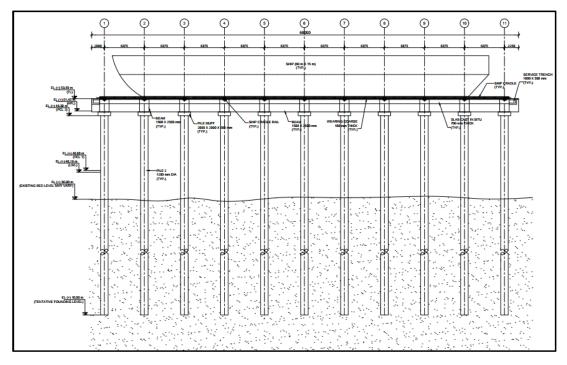


Fig. 6.6: Cross-section of Ship repair bay

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#### **6.2 Basic Design considerations**

#### 6.2.1 General

The facilities for the berthing structure consist of the following.

- The length of the boat hoist jetty is 60m and the width is 10m.
- The length of the tranferbay is 68m and the width is 37.5m.
- The length of the repairbay is 68m and the width is 32.5m.
- The jetty structure consists of a beam deck system and it is supported on pile.
- The ship repair facility is designed for various loads as described below.

#### 6.2.2 Design Life

This design life criterion is based on the maintenance system as follows:

- The design life of the proposed new facility is assumed as 50 years.
- The frequency of inspection & repairs of work is 5 years.
- The design life of the fender is 15 years.
- The design life of the bollard is 10 years.

#### 6.2.3 Design assumptions

Following assumptions were considered for the design of the berthing structure.

- While berthing the vessel, the berthing contact to the structure is quarter-point (L/4) berthing and the energy has been distributed to the structure as per code.
- Berthing shall be allowed only after the deck beam achieves its full strength for 28 days.
- The design shall be carried out considering soil parameters at the structure location. The static capacity of the pile is derived based on IS 2911. The following points have been taken into consideration for designing the piles.
  - a) A minimum factor of safety of 2.5 will be considered for arriving at the safe pile bearing capacity.

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#### 6.2.4 Design Loads

The ship repair facility structure shall be designed adequately for the following loads and appropriate combinations of the loads.

- Dead loads
- Live loads
- Berthing Force
- Mooring Force
- Boat Hoist Crane
- Seismic Force
- Temparature
- Wave and current force

#### 6.2.5 Design deck Level

The design deck level for the jetty is taken as (+) 52.0m CD.

## **6.2.6 Berthing Criteria**

The berth is located in a river location. Hence, the berthing condition is assumed in moderate conditions. The design berthing velocities to be adopted during the design of berthing structures are summarized in **Table**. **6.1**.

**Table. 6.1: Berthing Criteria** 

Items	1260DWT
Berthing Velocity (m/sec)	0.45
Berthing angle (deg)	10

#### **6.2.7 Mooring Loads**

Mooring conditions define the prevailing wind and current at the berth location during the presence of the ship. Following conditions shall be assessed.

Wind: 49 m/sec

Current: 3m/sec

Mooring bollard loads shall be arrived as per IS 4651. References shall be made to the report for details and the mooring bollard capacity of 30T is provided for the berthing structure.

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#### 6.2.8 Structural Design

The main considerations for the design of structures are:

- a. Structural safety, stability & durability.
- b. Availability of material, equipment and expertise.
- c. Constructability and ease of maintenance.
- d. Maximum DWT of the ship, which will be berthed during the lifetime of the structure.

#### 6.29 Materials and Cover

#### **6.2.9.1** Concrete

For all structural elements, an M40 grade concrete mix can be adopted.

#### 6.2.9.2 Reinforcement

High yield strength deformed bars of Fe 500D grade according to IS 1786-(2008) can be used. **Table. 6.2** shows the list of concrete and steel grades to be adopted for structural elements.

Table. 6.2: Grade of concrete & steel

Structures	Materials	
	Concrete	Steel
Pile	M40	Fe500D
Beam	M40	Fe500D
Slab	M40	Fe500D

#### 6.2.9.3 Cement

For all structural elements, OPC 53- grade cement according to IS: 12269 or 43 –grade cement according to IS: 8112 can be used.

#### 6.2.9.4 Clear Covers

The following clear cover shall be adopted for the outermost reinforcement of the structural members.

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Pile	75mm
Pilemuff	60mm
Beam	50mm
Slab	50mm

### 6.2.9.5 Vessel Specifications

The following table shows the vessel specifications which are going to be handled in the new proposed structure.

Table. 6.3: Vessel specifications

Description	Vessel
Vessel capacity	1260DWT
Draft	1.2m or 1.8m
Overall Length	90m
Width	15m

#### 6.2.9.6 Deflection

Deflection due to all loads including creep and shrinkage should not exceed span/250 as given in Clause 23.2 of IS 456:2000 and also as per client requirements.

#### 6.2.9.7 Crack Width

Crack width is checked under IS 4651 (Part 4)-2014. As a guide, assessed surface width of cracks at points nearest to the main reinforcement should not exceed 0.004 times the cover of the main reinforcement or maximum crack width in different zones are given in **Table. 6.4,** whichever is minimum. Limit state of serviceability load combinations is considered for crack width calculation.

Table. 6.4: Maximum Crack Width in Different Zone

SI	Exposure Zone	Maximum Crack Width	
No. (1)	(2)	Sustained Load (3)	Transient Load
i)	Atmospheric zone — above splash zone and where direct wave or spray impragement is infrequent	0.2	0.3
ii)	Splash zone — zone between the chart datum and the design wave height above the mean high water springs	0.1	0.2
iii)	Continuous seawater immersion zone — below splash zone upto seabed level	0.2	0.3
ive	Below seabed level	0.3	0.3

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### 7.0 General Arrangement and Structural Model for Proposed Structure

The ship repair facility consist of boat hoist jetty, transfer bay and repair bay. The proposed boat hoist jetty length is 60m and width is 10m. The transfer bay length is 68m and width is 37.5m. The repair bay length is 68m and width is 32.5m.

#### 7.1 Structural Arrangement of boat hoist jetty, transfer bay and repair bay.

The proposed cross section of boat hoist jetty consists of open piled type of jetty and the pile diameter is 1.3m. The length of jetty is 60m and width is 10m. The transvers and longitudinal beam dimensions are 1.75m x 2m and 1.5m x 2m. The centre to centre pile spacing is 6.16m in longitudinal direction and 8.25m in transvers direction. The adopted thickness of deck slab is 700mm and the wearing coat of 100mm is provided. The deck level of jetty is +53.50m (CD). The founding level of pile is +10m(CD). The plan view and cross-section of the typical jetty is shown in **Fig. 7.1** and **7.2** respectively.

The proposed cross section of transfer bay consists of open piled type of jetty and the pile diameter is 1.3m. The length of jetty is 68m and width is 37.5m. The transvers and longitudinal beam dimensions are 1.75m x 2m and 1.5m x 2m. The centre to centre pile spacing is 6.375m in longitudinal direction and 7.625m and 9.625m in transvers direction. The adopted thickness of deck slab is 800mm and the wearing coat of 100mm is provided. The deck level of jetty is +53.50m (CD). The founding level of pile is +10m(CD). The plan view and cross-section of the typical jetty is shown in **Fig. 7.3** and **7.4** respectively.

The proposed cross section of repair bay consists of open piled type of jetty and the pile diameter is 1.2m. The length of jetty is 68m and width is 32.5m. The transvers and longitudinal beam dimension is 1.5m x 2m. The centre to centre pile spacing is 6.375m in longitudinal direction and 7.0m in transvers direction. The adopted thickness of deck slab is 700mm and the wearing coat of 100mm is provided. The deck level of jetty is +53.50m (CD). The founding level of pile is +10m(CD). The plan view and cross-section of the typical jetty is shown in **Figure. 7.5** and **7.6** respectively.

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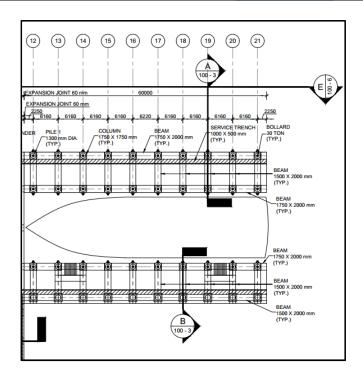


Fig. 7.1: Plan view of boat hoist jetty

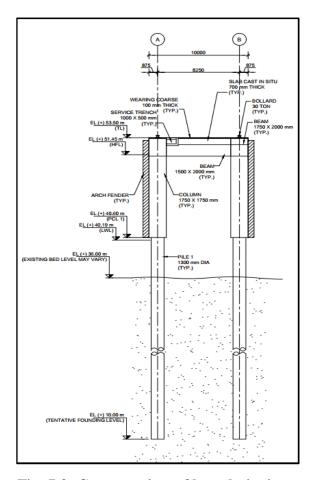


Fig. 7.2: Cross-section of boat hoist jetty

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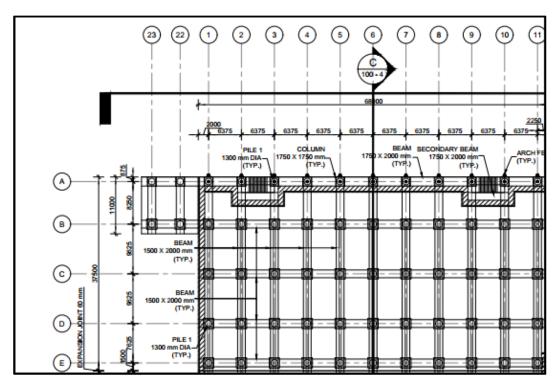


Fig. 7.3: Plan view of the transfer bay

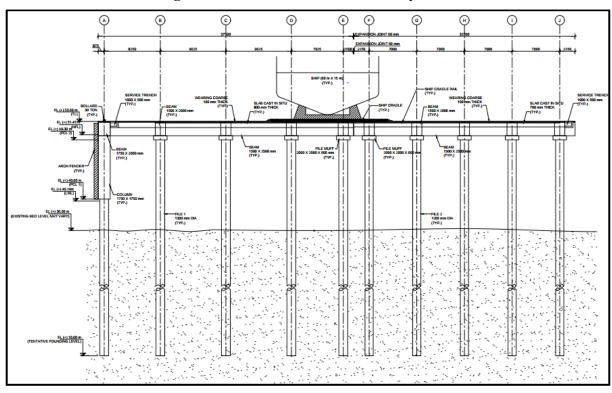


Fig. 7.4: Cross-section of transfer bay

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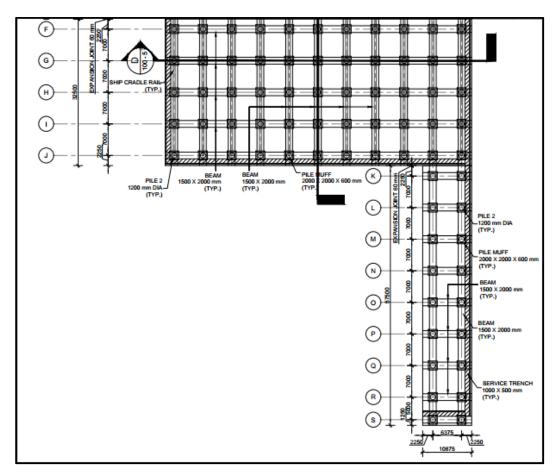


Fig. 7.5: Plan view of the ship repair bay

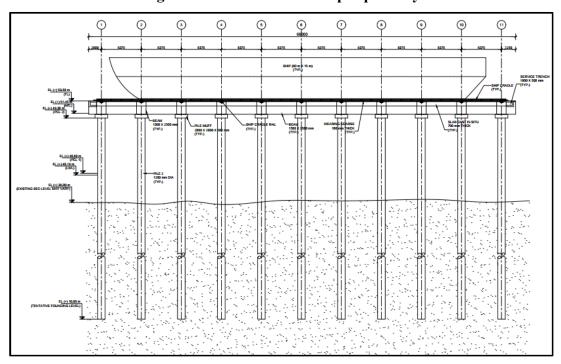


Fig. 7.6: Front-section of Ship repair bay

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#### 7.2 Analysis Model

The computer model of boat hoist jetty, transfer bay, repair bay and approach structure are considered as a three-dimensional frame that includes the pile and beam along with provided soil springs. The structure is modeled as a single unit. The pile is supported by soil springs. The pile is modeled as beam elements with appropriate properties of RCC sections. The slab modeled as plate elements and it is supported by beam elements. This represents the stiffness of the structure and soil simulated. The analysis is carried out using the conventional STAAD ProV8i package for various critical load combinations. The P-δ analysis is carried out; Soil-Structure interaction analysis is performed by using springs as a representation of soil. The boat hoist jetty structural model is shown in **Fig. 7.7** and transfer bay is shown in **Fig. 7.8**. The repair bay and approach structural model is shown in **Fig. 7.9**.

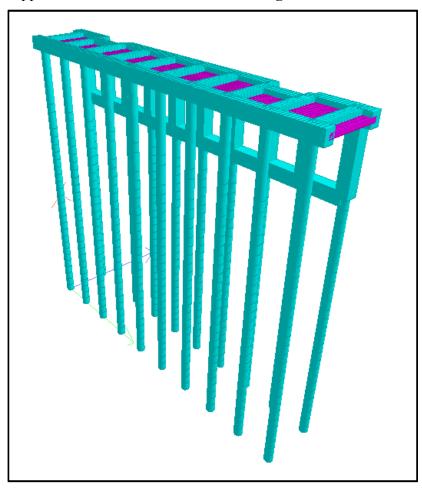


Fig. 7.7: Structural model for boat hoist jetty

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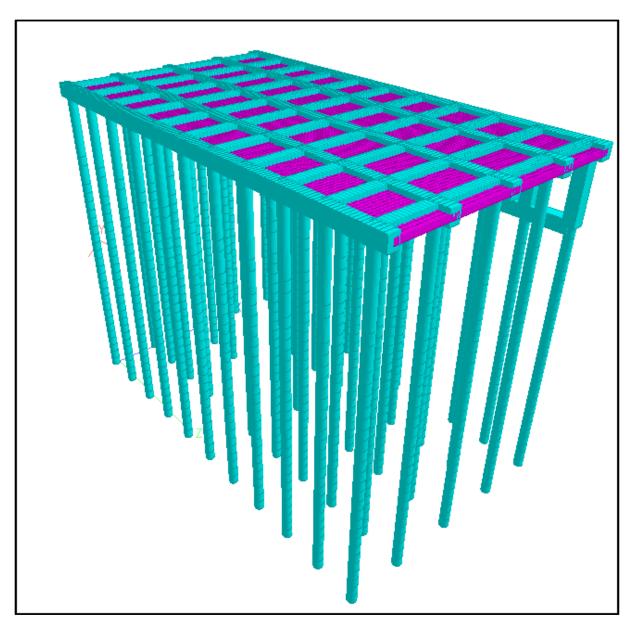


Fig. 7.8: Structural model for transfer bay

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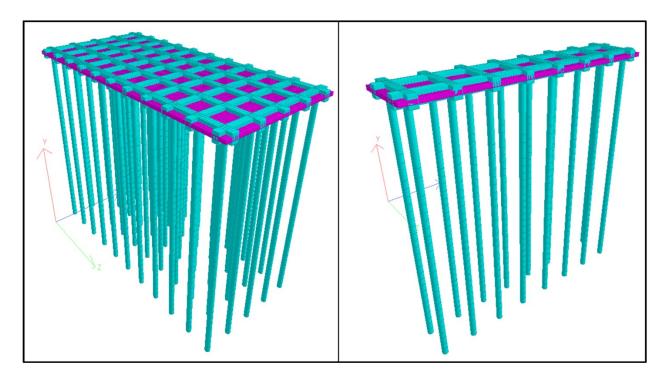


Fig. 7.9: Structural model for repair bay

#### 7.3 Structural Sizes

The proposed structure is a RCC-framed structure and the proposed sizes are summarised in **Table. 7.1**. These sizes are used in the simulation for structural analysis to generate the dead loads.

**Table. 7.1: Structural components** 

S: No	Structural member	Dimension
	heet pile wall jetty	
1	Piles	1300mm&1200mm
2	Beams	1750mmx2000mm&1500mmx2000mm
3	Slab	700mm & 800mm
4	Pilemuff	2000mmx2000mmx600mm

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#### 8.0 Loadings for Ship repair facilitates

#### 8.1 Dead Loads

Dead loads will consist of the weights of structure and all equipment of a permanent or semi-permanent nature. Unit weight of various materials used in the structural members is given as follows:

a.	Water	= 1.0  T/cum
b.	Sea water	= 1.03  T/cum
c.	RCC	= 2.5 T/cum
d.	Soil	= 1.8 T/cum
e.	Steel	= 7.85 T/cum
f.	Sand	= 2.0 T/cum
g.	Concrete pavers	= 2.4 T/cum
h.	Foam Concrete	=0.6 T/cum

#### 8.2 Live Load

A live load of 50kN/ m2 is adopted for design.

#### 8.3 Seismic Force

Pandu is in **Zone V** of the Indian Map of Seismic zones (IS-1893 Part-1 2016), which is a severe risk of the seismic zone. Seismic force is calculated according to IS 1893-2002, considering 100 % dead load + 50 % live load acting on the structure. As per IS code, the site is under (Zone III) and the basic horizontal seismic coefficient is calculated accordingly. The seismic force calculations are given below. **Fig. 8.1** shows the response spectra for rock and soil and sites for 5% damping.

Horizontal seismic coefficient

Ah = Z I Sa/g

Where,

Z = 0.36, Zone factor (Table 2 of IS: 1893-2016)

I = 1.5, Importance factor (Table 6 of IS: 1893-2016)

R = 3.0, Response reduction factor (Table 7 of IS: 1893-2016)

 $Sa/g = \overline{A}$ verage Response acceleration coefficient

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(Depends on the natural time period of the structure from STAAD Pro analysis).

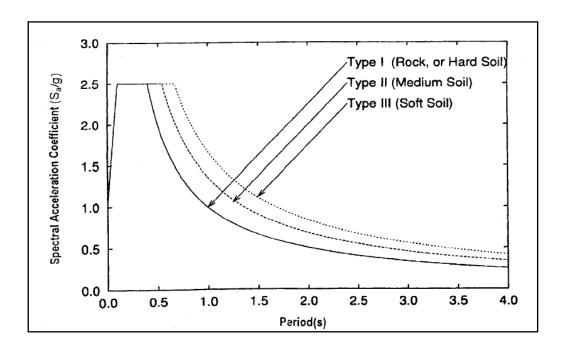


Fig. 8.1: Response spectra for rock and soil and sites for 5% damping

## 8.4 Berthing Force

It is proposed to handle 1260 DWT vessels and the design of the berthing structure considers under moderate conditions. Berthing loads shall be determined under IS 4651-III (2020) for the design vessels. Approach velocities and angles of approach for design vessels shall be considered as per the table given below.

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Table. 8.1: Parameters for calculation of berthing energy

S No	Description	Vessel Size in 1260 DWT
1	Berthing velocity (m/sec)	0.45
2	Berthing angle (degrees)	10
3	Mass Coefficient	1.21
4	Displacement Tonnage (Tonne)	1665
5	Eccentricity Coefficient	0.52
6	Softness Coefficient	0.9
7	Normal berthing Energy (Tonne.m)	10.1
8	Factor of Safety	2
9	Abnormal berthing Energy (Tonne.m)	20.2
10	Design berthing Energy (Tonne.m)	22.01
11	Fender Selected	Trelleborg Arch Fender- 1000(E1.5)
12	Maximum Energy Absorption (Tonne.m)	22.1
13	Maximum Rated Reaction (Tonne)	567

## 8.5 Mooring Force

## Vessel specifications

Dead Weight Tonnage	DT	=	1665	T
Overall Length	LOA	=	90	m
Length between perpendiculars	$L_{p}$	=	85	m
Width of the vessel	В	=	15	m
Moulded depth	$D_{m}$	=	2.5	m
Average light draft	$D_{l}$	=	1.2	m
Other data				
Basic Wind Speed	$V_b$	=	47	m/s
Current velocity	V	=	3	m/s

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Unit weigth of sea water	W	=	10.25	kN/m
Acceleration due to gravity	g	=	9.81	$m/s^2$
No. of mooring lines	8	=	8	
No. of mooring lines resisting the system		=	2	
Coefficients				
Shape Factor	$C_{\rm w}$	=	1.3	
Coefficients for wind force calculation	- "			
Probability factor	$\mathbf{k}_1$	=	1	
Terrain roughness and height factor	$k_2$	=	1.05	
Topography factor	k <sub>3</sub>	=	1	
Importance factor	k <sub>4</sub>	=	1	
Wind directionality factor	K <sub>d</sub>	=	1	
Area averaging factor	Ka	=	0.8	
Combination factor	Kc	=	0.9	
Force due to wind				
Force due to wind	$F_{\mathbf{w}}$	=	$C_wA_wP$	
Shape Factor	$C_{\mathrm{w}}$	=	1.3	
Windage Area	$A_{\rm w}$	=	$1.175*L_p(D_m-D_l)$	
		=	1.175*85(2.5-1.2)	
	$A_{\rm w}$	=	129.84	$m^2$
Design Wind Speed	$V_{z}$	=	Vb*k1*k2*k3*k4	
		=	47*1*1.05*1*1	
	$V_{z}$	=	49.35	m/s
Wind Pressure	$P_z$	=	$0.6*V_z^2$	
		=	0.6*49.35^2	
	$P_z$	=	1461.250	$N/m^2$
Design wind pressure	pd	=	$K_d*K_a*K_c*p_z$	
		=	1*0.8*0.9*1461.25	

The value of  $p_{\text{d}},$  however shall not be taken less than  $0.70~p_{z}$ 

 $p_{d}$ 

$$0.7p_z = 0.7*1461.25$$
  
= 1022.88

= 1052.1

 $N/m^2$ 

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Wind pressure to be considered,		=	1052.1	$N/m^2$
	P	=	1.05	$kN/m^2$
Force due to wind	$F_{\boldsymbol{w}}$	=	$C_wA_wP$	
		=	1.3*129.838*1.05	
		=	177.23	kN
Force due to current				
Force due to current	$F_{c}$	=	$(wv^2/2g)*A_p$	
Area of ship under water	$A_p$	=	$B*D_1$	
		=	15*1.2	
		=	18	$m^2$
Force due to current	$F_{c}$	=	$(wv^2/2g)*A_p$	
		=	((10.25*3^2)/(2*9.81))*18	
			84.63	kN
Resultant Mooring force	F	=	$(F_w^2 + F_c^2)^{0.5}$	
		=	(177.23^2 + 84.63^2)^0.5	
Resultant Mooring force	F		196.40	kN
Assuming vessel to be moored by 8 moore resist the force	ing line	s ana	l considering only 50 % of the	lines
Mooring force on each mooring line		=	196.4 / 2	
			98.20	kN
		~	300	kN

#### **8.6 Wind**

The wind forces on structures are calculated as per IS 875 Part 3 with the following wind speed consideration (assumed).

The basic maximum wind speed under the operating condition: 47 m/s.

The design wind speed to be obtained from the formula is given below:

Design wind speed,  $Vz = k1 \times k2 \times k3 \times Vb$ 

Where,

k1 = Risk Co efficient = 1.06 (Table 1),

k2 = Terrain, Height and Structure size factor = 1.05 (Category 2, Class C, Height < 10m (Table 2)),

k3 = Topography factor = 1.0 (Clause: 5.3.3.1).

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### 8.7 Temperature & Shrinkage

Temperature and Shrinkage forces considered for adequacy check are as follows.

• Min / Max temperature

 $: + 5^{\circ}C / 40^{\circ} C$ 

• The coefficient of thermal expansion for RCC structures is taken as  $11.7 \times 10-6 / ^{\circ}$ C.

#### 8.8 Boat Hoist Crane

The ship repair structure is designed for 800T capacity of boat hoist carne. The technical details are furnished in **Fig. 8.2**. The customized crane should have 32wheels with spacing of 4.9m centre to centre.

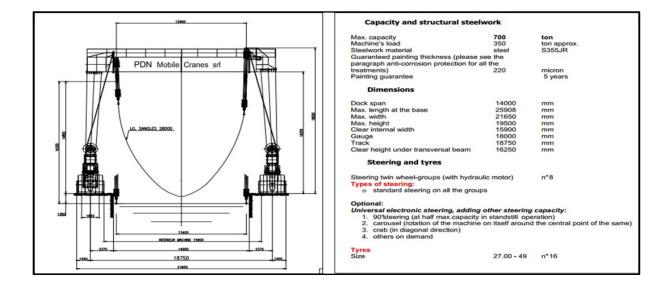


Fig. 8.2: Technical data

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	SPRING VALUE CALCULATION FOR CIRCULAR SECTION											
	General Da											
			concrete	=	40	MPa		Soil level	36	from CD	T	1
S.No.	Soil Level	Segment length	Wall thickness	Poisson ratio for soil	Elastic modulus of Pile	Moment of Inertia	N	Elastic modulus of soil	$\mathrm{E_s/(1\text{-}m_s^{\ 2})}$	${(E_sD^4/\!(E_p\;I_p))}^{^{^{^{^{^{^{^{^{^{^{^{^{^{}}}}}}}}}$	Modulus of subgrade reaction	Spring value
	Тор	L	B or D	$\mu_{s}$	$E_p = 5000 (f_{ck})^{^{\circ}0.5}$	$I_p$		E <sub>s</sub>			k <sub>s</sub>	
	m	m	m		kN/m <sup>2</sup>	m <sup>4</sup>		KPa			kN/m <sup>3</sup>	kN/m
1	36.000	1.50	1.3	0.25	31622776.6	0.1401	10	49030	52299	0.75	39216.3	37540.1
2	34.500	1.50	1.3	0.25	31622776.6	0.1401	10	49030	52299	0.75	39216.3	77863.4
3	33.000	1.50	1.3	0.25	31622776.6	0.1401	10	58836	62758	0.76	47780.0	92482.7
4	31.500	1.50	1.3	0.25	31622776.6	0.1401	10	63739	67988	0.77	52108.1	102327.4
5	30.000	1.50	1.3	0.25	31622776.6	0.1401	10	73545	78448	0.78	60846.0	117945.9
6	28.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	126526.2
7	27.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
8	25.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
9	24.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
10	22.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
11	21.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
12	19.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
13	18.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
14	16.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
15	15.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
16	13.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
17	12.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
18	10.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
19	9.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
20	7.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
21	6.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
22	4.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
23	3.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
24	1.500	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	127242.2
25	0.000	1.50	1.3	0.25	31622776.6	0.1401	10	78448	83678	0.78	65252.4	63621.1

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#### 8.10 Load Combinations

Load Combination factors for the analysis are under IS 4651(Part 4)-2014. The following load combination has been considered in the analysis.

### Limit state of serviceability

1DL+1.1LL+1.1DYN+1.0WF+1.0B.F

1DL+1.1LL+1.1DYN+1.0WF+1.0M.F

1DL+1.1LL+1.1DYN+1.0WF+1.0S.F

1DL+0.5LL+0.5DYN+1.0TEM

### Limit state of collapsibility

1.5DL+1.5LL+1.5DYN+1.2WF+1.5B.F

1.5DL+1.5LL+1.5DYN+1.2WF+1.5M.F

1.5DL+1.5LL

1.2DL+1.2LL+1.2S.F

1.5DL+1.5S.F

1.2DL+1.2LL+1.2DYN+1.0WF+1.0B.F or M.F

1.2DL+1.2LL+1.2DYN+1.0WF+1.2S.F

1.2DL+1.2LL+1.2DYN+1.0WF+1.2T.F

0.9DL+0.9LL+0.9DYN+1.0WF+1.5B.F or M.F

0.9DL+0.9LL+0.9DYN+1.0WF+1.5S.F

#### Where,

D.L - Dead Load

L.L - Live Load

M.F - Mooring Force

B.F - Berthing Force

S.F - Seismic Force

DYN - Vehicle load

T.F - Temperature

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### 9.0 Structural Analysis Results

#### 9.1. Pile-1300mm dia

### Limit state of collapse

## **Beam Force Detail Summary**

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	Shear		Torsion	Ben	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kNm)	(kN <sup>-</sup> m)
Max Fx	3967	2056:NORMAL	1.500	11.3E+3	0.006	0.000	-0	-0.000	0.000
Min Fx	1633	3499:REVERS	0	-2.78E+3	-991.443	1.85E+3	0.000	-7.3E+3	-922.835
Max Fy	1635	3081:REVERS	3.100	5.81E+3	1.67E+3	-445.427	0.000	-273.123	-6.09E+3
Min Fy	1635	3283:REVERS	3.100	-826.459	-1.54E+3	416.338	-0.000	293.700	5.91E+3
Max Fz	2924	3459:REVERS	3.100	3.2E+3	278.402	2.08E+3	0.000	1.02E+3	-870.611
Min Fz	3068	3696:REVERS	3.100	2.93E+3	-221.539	-2.08E+3	-0.000	-1.04E+3	1.09E+3
Max Mx	4006	3477:REVERS	0	5.05E+3	12.787	-102.193	0.000	90.945	49.997
Min Mx	4006	3679:REVERS	0	2.69E+3	-13.016	100.783	-0.000	-89.037	-50.992
Max My	3066	3696:REVERS	0	2.84E+3	-215.517	-2.06E+3	-0.000	8.5E+3	84.501
Min My	2922	3459:REVERS	0	3.11E+3	272.379	2.06E+3	0.000	-8.5E+3	396.184
Max Mz	1637	3283:REVERS	1.500	-783.544	-1.26E+3	245.380	-0.000	659.654	7.79E+3
Min Mz	1637	3081:REVERS	1.500	5.86E+3	1.37E+3	-266.063	0.000	-670.101	-8.14E+3

## Limit state of Serviceability

### Beam Force Detail Summary

Sign convention as diagrams: - positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	She	ear	Torsion	Ben	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kN'm)	(kN <sup>-</sup> m)
Max Fx	3967	112:SHORT-CF	1.500	7.53E+3	0.005	0.000	-0	-0.000	0.000
Min Fx	2	229:SHORT-CF	0	941.588	-4.869	-3.476	-0.000	43.507	855.435
Max Fy	3066	228:SHORT-CF	0	4.23E+3	513.681	-4.201	-0.000	18.490	652.782
Min Fy	3786	67:SHORT-CR/	0	1.85E+3	-414.315	23.020	-0.000	-105.340	-457.831
Max Fz	3498	72:SHORT-CR/	0	3.62E+3	130.914	464.276	0.000	-1.92E+3	301.461
Min Fz	3498	171:SHORT-CF	0	2.83E+3	23.216	-475.997	-0.000	1.95E+3	109.969
Max Mx	4034	165:SHORT-CF	0	4.96E+3	-1.716	-23.021	0.000	20.956	-7.457
Min Mx	3659	79:SHORT-CR/	0	3.05E+3	3.464	17.949	-0.000	-15.996	15.053
Max My	3498	171:SHORT-CF	0	2.83E+3	23.216	-475.997	-0.000	1.95E+3	109.969
Min My	3498	72:SHORT-CR/	0	3.62E+3	130.914	464.276	0.000	-1.92E+3	301.461
Max Mz	3790	67:SHORT-CR/	1.500	2E+3	-339.589	14.876	-0.000	22.868	1.96E+3
Min Mz	2638	225:SHORT-CF	1.500	4.33E+3	417.090	-4.421	0.000	-11.632	-2.34E+3

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#### **Beam Displacement Detail Summary** Displacements shown in italic indicate the presence of an offset Beam L/C Z Resultant (m) (mm) (mm) (mm) (mm) 225:SHORT-CF 3726 0.496 23.755 Max X 23.221 -4.98621.994 Min X 3726 65:SHORT-CR/ 1.015 -21.235 -5.625-1.096Max Y 54 57:SHORT-CR/ 1.500 -0.000 0 0 0.000 Min Y 3941 112:SHORT-CF 0 -19.813 -11.804 0.057 23.063 Max Z 3582 165:SHORT-CF 0 10.728 -5.628 24.910 27.700 Min Z 3582 79:SHORT-CR/ 0 -9.238 -3.705 25.529 Max Rst 165:SHORT-CF 0 24.910 28.625 3726 13.042 -5.367

#### 9.2. Pile-1200mm dia

#### Limit state of collapse

## **Beam Force Detail Summary**

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	Sh	ear	Torsion	Ben	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kN'm)	(kN <sup>-</sup> m)
Max Fx	3967	2013:NORMAL	1.500	8.4E+3	0.000	0.011	-0	-0.000	0.000
Min Fx	1631	3497:REVERS	0	18.220	-152.942	369.569	0.000	-4.6E+3	-1.25E+3
Max Fy	3886	3081:REVERS	3.100	3.76E+3	732.416	-140.490	-0.000	-755.476	-4.72E+3
Min Fy	3886	3280:REVERS	3.100	3.12E+3	-732.803	119.946	0.000	696.006	4.72E+3
Max Fz	3448	3585:REVERS	0	2.68E+3	32.433	678.825	-0.000	-2.78E+3	39.699
Min Fz	3016	3496:REVERS	0	3.06E+3	-40.757	-678.616	0.000	2.78E+3	-49.525
Max Mx	2	3183:REVERS/	0	2.84E+3	143.043	-403.893	0.000	3.81E+3	2.12E+3
Min Mx	2	3274:REVERS	0	2.34E+3	-163.980	384.926	-0.000	-3.59E+3	-2.34E+3
Max My	3438	3585:REVERS	0	2.32E+3	105.365	-416.052	-0.000	5.1E+3	876.457
Min My	3006	3496:REVERS	0	2.73E+3	-135.268	418.232	0.000	-5.14E+3	-1.14E+3
Max Mz	3887	3280:REVERS	1.500	3.16E+3	-549.828	3.837	0.000	700.216	5.54E+3
Min Mz	3887	3081:REVERS	1.500	3.8E+3	549.542	-10.459	-0.000	-769.619	-5.54E+3

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### Limit state of Serviceability

## **Beam Force Detail Summary**

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	She	ear	Torsion	Bene	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kN'm)	(kN <sup>-</sup> m)
Max Fx	3967	69:SHORT-CR/	1.500	5.6E+3	0.000	0.009	-0	-0.000	0.000
Min Fx	2	239:SHORT-CF	0	1.18E+3	-4.345	-7.282	-0.000	-1.06E+3	-47.316
Max Fy	3208	226:SHORT-CF	4.060	2.17E+3	188.382	2.348	0.000	-23.817	825.747
Min Fy	3583	64:SHORT-CR/	4.060	2.5E+3	-188.935	0.639	0.000	-9.043	-831.186
Max Fz	13	75:SHORT-CR/	0	4.01E+3	-1.187	192.659	-0.000	-794.868	-1.401
Min Fz	3074	69:SHORT-CR/	0	2.73E+3	2.932	-194.082	-0.000	803.181	3.461
Max Mx	1632	117:SHORT-CF	0	3.19E+3	4.275	-14.792	0.000	-966.825	46.312
Min Mx	1632	244:SHORT-CF	0	3.63E+3	3.618	-7.346	-0.000	1.23E+3	40.990
Max My	3884	75:SHORT-CR/	0	3.75E+3	0.095	-7.491	-0.000	1.23E+3	0.793
Min My	3064	69:SHORT-CR/	0	2.35E+3	10.021	12.174	-0.000	-1.3E+3	113.645
Max Mz	2724	111:SHORT-CR	1.500	4E+3	-140.603	0.407	-0.000	-6.067	1.4E+3
Min Mz	3214	226:SHORT-CF	1.500	2.42E+3	140.639	1.324	0.000	3.265	-1.4E+3

## **Beam Displacement Detail Summary**

Displacements shown in italic indicate the presence of an offset

	Beam	L/C	d	X	Y	Z	Resultant
			(m)	(mm)	(mm)	(mm)	(mm)
Max X	3726	154:SHORT-CF	0	27.752	-3.848	-0.267	28.019
Min X	3784	63:SHORT-CR/	0	-28.206	-3.804	-0.167	28.462
Max Y	54	57:SHORT-CR/	1.500	0	0	0	0
Min Y	3941	69:SHORT-CR/	0	-0.099	-10.127	38.933	40.229
Max Z	2	167:SHORT-CF	0	0	-3.702	39.250	39.425
Min Z	3006	75:SHORT-CR/	0	-0.268	-7.103	-39.638	40.270
Max Rst	3969	76:SHORT-CR/	0	-0.037	-10.015	-39.528	40.777

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#### 9.3. Column-1750x1750mm

## Limit state of collapse

## **Beam Force Detail Summary**

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	She	ear	Torsion	Ben	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN'm)	(kN'm)	(kN <sup>-</sup> m)
Max Fx	1688	2309:EXTREME	10.150	5.65E+3	1.35E+3	-426.584	194.398	-2.06E+3	1.31E+3
Min Fx	1631	3283:REVERS/	0	-1.01E+3	-1.14E+3	142.952	-234.318	-977.674	-14.2E+3
Max Fy	1631	3081:REVERS	10.150	4.66E+3	1.54E+3	-370.103	247.130	-1.55E+3	1.26E+3
Min Fy	1631	3283:REVERS/	10.150	-555.759	-1.42E+3	172.533	-234.318	623.412	-808.967
Max Fz	2920	3423:REVERS	10.150	2.79E+3	228.816	2.31E+3	-871.071	11.1E+3	439.622
Min Fz	3208	3696:REVERS	10.150	2.24E+3	-300.142	-2.26E+3	782.808	-10.8E+3	13.032
Max Mx	2632	3678:REVERS	0	3.27E+3	734.027	-1.88E+3	840.926	10.6E+3	8.28E+3
Min Mx	2776	3477:REVERS	0	1.63E+3	-401.123	2.01E+3	-904.895	-11.3E+3	-4.33E+3
Max My	3208	3696:REVERS	0	1.79E+3	-270.561	-1.99E+3	782.808	11.1E+3	-2.88E+3
Min My	2920	3423:REVERS	0	2.3E+3	199.235	2.04E+3	-871.071	-11.4E+3	2.61E+3
Max Mz	1688	3081:REVERS	0	4.58E+3	1.26E+3	-450.913	245.539	2.41E+3	15.9E+3
Min Mz	1631	3283:REVERS/	0	-1.01E+3	-1.14E+3	142.952	-234.318	-977.674	-14.2E+3

## Limit state of Serviceability

## **Beam Force Detail Summary**

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	Shear		Torsion Ben		ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN'm)	(kNm)	(kN <sup>-</sup> m)
Max Fx	3352	228:SHORT-CF	10.150	3.81E+3	519.572	18.248	-37.852	105.701	582.073
Min Fx	1631	115:SHORT-CF	0	869.249	-95.775	-71.157	3.659	365.238	-2.37E+3
Max Fy	3352	132:SHORT-CF	0	3.16E+3	808.431	26.756	-21.582	-124.165	4.71E+3
Min Fy	3640	90:SHORT-CR/	9.135	1.89E+3	-703.852	75.574	-23.510	303.809	-719.354
Max Fz	3496	72:SHORT-CR/	4.060	2.91E+3	152.258	637.445	-240.222	-914.394	1.18E+3
Min Fz	3496	171:SHORT-CF	4.060	2.04E+3	23.095	-598.410	120.479	839.220	260.486
Max Mx	3784	101:SHORT-CF	0	2.36E+3	3.827	22.683	297.192	547.565	15.912
Min Mx	3496	96:SHORT-CR/	0	2.74E+3	225.883	450.333	-336.649	-2.95E+3	2.21E+3
Max My	3496	72:SHORT-CR/	10.150	3.23E+3	152.258	637.445	-240.222	2.97E+3	255.823
Min My	3496	72:SHORT-CR/	0	2.7E+3	152.258	459.695	-240.222	-3E+3	1.8E+3
Max Mz	2632	225:SHORT-CF	0	3.2E+3	348.033	-1.009	-38.550	4.769	5.42E+3
Min Mz	3784	67:SHORT-CR/	5.075	1.26E+3	173.301	66.043	-120.901	-0.743	-2.75E+3

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# **Beam Displacement Detail Summary**

Displacements shown in italic indicate the presence of an offset

	Beam	L/C	d	Х	Y	Z	Resultant
			(m)	(mm)	(mm)	(mm)	(mm)
Max X	2430	153:SHORT-CF	3.045	10.422	-8.395	-0.283	13.386
Min X	1557	65:SHORT-CR/	3.045	-10.984	-9.615	-0.435	14.604
Max Y	1631	120:SHORT-CF	10.150	-0.05271	-2.201	3.088	3.793
Min Y	2430	119: SHORT-CF	0	-0.440	-10.750	5.017	11.871
Max Z	1224	167:SHORT-CF	1.015	0.825	-9.029	6.486	11.148
Min Z	1335	78:SHORT-CR/	0	-2.365	-5.547	-6.003	8.508
Max Rst	1557	65:SHORT-CR/	2.030	-10.887	-9.734	-0.387	14.609

#### 9.4. Beam-1500x2000mm

## Limit state of collapse

## **Beam Force Detail Summary**

Sign convention as diagrams: - positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	She	ear	Torsion	Ben	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kNm)	(kN <sup>-</sup> m)
Max Fx	14454	3083:REVERS	0.412	662.448	-2.37E+3	-113.542	544.510	-145.150	12.5E+3
Min Fx	14454	3283:REVERS/	0.412	-604.804	1.13E+3	83.538	-219.329	117.992	-11.6E+3
Max Fy	14382	2493:EXTREME	0	-135.841	1.85E+3	-2.441	172.620	1.339	-1.98E+3
Min Fy	14454	2309:EXTREME	0.412	565.121	-2.41E+3	-101.719	584.349	-124.194	10.8E+3
Max Fz	14454	3499:REVERS/	0.412	-392.323	496.615	262.336	-874.540	345.300	-6.3E+3
Min Fz	14454	3588:REVERS/	0.412	447.928	-1.74E+3	-292.912	1.2E+3	-372.807	7.15E+3
Max Mx	14533	3298:REVERS/	0	29.010	-951.626	-124.283	1.87E+3	-21.519	434.620
Min Mx	14616	3063:REVERS	0	-48.536	-1.04E+3	138.231	-1.86E+3	29.322	546.363
Max My	14454	3499:REVERS/	0.412	-392.323	496.615	262.336	-874.540	345.300	-6.3E+3
Min My	14454	3588:REVERS/	0.412	447.928	-1.74E+3	-292.912	1.2E+3	-372.807	7.15E+3
Max Mz	14454	3081:REVERS	0.412	661.895	-2.38E+3	-113.550	556.188	-143.496	12.5E+3
Min Mz	14454	3283:REVERS	0.412	-604.804	1.13E+3	83.538	-219.329	117.992	-11.6E+3

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## Limit state of Serviceability

## **Beam Force Detail Summary**

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	Sh	ear	Torsion	Ben	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kN'm)	(kN <sup>-</sup> m)
Max Fx	3529	195:SHORT-CF	0	516.495	-193.187	15.319	-2.022	35.359	115.097
Min Fx	2343	148:SHORT-CF	0	-442.783	353.547	24.751	52.592	-66.249	443.118
Max Fy	112	135:SHORT-CF	0	-9.044	1.49E+3	8.788	45.496	-15.448	689.281
Min Fy	4232	201:SHORT-CF	0.500	-74.915	-1.75E+3	7.243	-11.847	-28.879	4.33E+3
Max Fz	4546	190:SHORT-CF	0	-122.077	-853.176	145.119	-175.920	106.669	1.07E+3
Min Fz	4546	123:SHORT-CF	0	-51.736	-811.005	-135.596	393.369	-123.386	-820.843
Max Mx	4546	219:SHORT-CF	0	-41.677	-951.776	-130.647	410.098	-132.640	34.066
Min Mx	4309	165:SHORT-CF	0	-54.134	-984.566	110.844	-410.921	125.718	-40.599
Max My	4309	166:SHORT-CF	0.500	-51.019	-729.000	124.257	-307.783	192.579	267.667
Min My	4546	219:SHORT-CF	0.500	-41.677	-987.118	-130.647	410.098	-197.964	518.790
Max Mz	4622	153:SHORT-CF	0.500	-80.078	-1.56E+3	24.518	0.651	63.363	4.41E+3
Min Mz	2812	135:SHORT-CF	0.521	-18.884	44.644	1.011	19.533	-1.243	-3.99E+3

## Beam Displacement Detail Summary

	Beam	L/C	d	Х	Y	Z	Resultant
			(m)	(mm)	(mm)	(mm)	(mm)
Max X	4653	202:SHORT-CF	0.512	39.129	-2.155	-2.754	39.285
Min X	2669	63:SHORT-CR/	0	-32.688	-3.665	0.105	32.893
Max Y	2670	154:SHORT-CF	0	38.947	-1.910	2.660	39.085
Min Y	2798	147:SHORT-CF	0.521	-1.518	-7.131	-9.534	12.002
Max Z	2670	166:SHORT-CF	0.250	23.733	-2.651	12.809	27.099
Min Z	3033	219:SHORT-CF	0.257	11.490	-4.294	-13.136	17.973
Max Rst	4653	202:SHORT-CF	0.512	39.129	-2.155	-2.754	39.285

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#### 9.5. Beam-1750x2000mm

## Limit state of collapse

## **Beam Force Detail Summary**

Sign convention as diagrams: - positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	She	ear	Torsion	Ben	ding
	Beam	L/C	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kN'm)	(kN m)
Max Fx	3695	3495:REVERS	6.375	958.160	-3.07E+3	-140.999	1.35E+3	-612.171	9.9E+3
Min Fx	2687	3423:REVERS	0	-1.15E+3	-2.66E+3	-134.173	1.3E+3	665.589	-9.85E+3
Max Fy	3551	3657:REVERS	0	-607.214	3.38E+3	123.075	-1.34E+3	-436.996	10.2E+3
Min Fy	3695	3426:REVERS	6.375	902.920	-3.37E+3	-189.222	1.24E+3	-716.229	10.9E+3
Max Fz	14766	3426:REVERS	0.563	-102.358	-229.088	280.795	-458.182	380.351	71.420
Min Fz	3695	2040:NORMAL	0	119.900	-701.105	-314.948	63.362	469.368	-2.84E+3
Max Mx	11 28 2	3066:REVERS	0	312.755	-1.18E+3	-189.574	1.91E+3	-82.740	1.19E+3
Min Mx	1685	3574:REVERS	0	186.766	1.12E+3	185.466	-2.07E+3	-124.407	2.39E+3
Max My	2503	3499:REVERS	0	-910.938	-2.54E+3	-155.092	1.04E+3	710.686	-11E+3
Min My	3695	2040:NORMAL	6.375	119.900	-1.49E+3	-314.948	63.362	-1.54E+3	4.14E+3
Max Mz	2503	3657:REVERS	0	757.077	3.25E+3	128.892	-956.733	-678.260	12.1E+3
Min Mz	3839	3696:REVERS	6.375	-870.790	2.69E+3	164.382	-975.746	677.977	-11.3E+3

## Limit state of Serviceability

## **Beam Force Detail Summary**

Sign convention as diagrams: - positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	She	ear	Torsion	Ben	ding
	Beam	ΓC	d	Fx	Fy	Fz	Mx	My	Mz
			(m)	(kN)	(kN)	(kN)	(kN m)	(kNm)	(kN <sup>-</sup> m)
Max Fx	10295	144:SHORT-CF	0	352.691	-898.781	-121.307	637.751	-65.543	1.12E+3
Min Fx	2687	69:SHORT-CR/	0	-379.851	-365.474	-25.361	258.532	183.558	-2.04E+3
Max Fy	2503	171:SHORT-CF	0	51.423	1.1E+3	36.221	-106.304	-120.842	3.43E+3
Min Fy	3695	72:SHORT-CR/	6.375	135.895	-1.14E+3	-48.145	180.228	-21.156	3.29E+3
Max Fz	3839	102:SHORT-CF	0	-164.160	959.200	191.005	-110.383	-940.223	2.08E+3
Min Fz	3695	96:SHORT-CR/	0	122.421	-605.784	-217.722	98.459	333.197	-2.31E+3
Max Mx	11 28 2	228:SHORT-CF	0	115.206	-785.425	2.780	953.241	6.539	-18.910
Min Mx	1685	244:SHORT-CF	0	109.595	893.788	-20.417	-844.391	36.088	1.56E+3
Max My	3695	77:SHORT-CR/	6.375	-275.904	362.366	57.050	-325.961	401.162	-1.9E+3
Min My	3695	96:SHORT-CR/	6.375	122.421	-1.13E+3	-217.722	98.459	-1.05E+3	3.23E+3
Max Mz	2503	171:SHORT-CF	0	51.423	1.1E+3	36.221	-106.304	-120.842	3.43E+3
Min Mz	3839	174:SHORT-CF	6.375	-176.158	488.776	37.367	-117.877	97.962	-2.57E+3

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eam [	)ispla	cement D	<u>etail Sı</u>	ummar	Y		
isplacemen	ts shown ii	n itali c indicate th	e presence (	of an offset			
	Beam	L/C	d	Х	Υ	Z	Resultan
			(m)	(mm)	(mm)	(mm)	(mm)
Max X	3870	225:SHORT-CF	0	23.267	-6.220	-0.242	24.08
Min X	3870	65:SHORT-CR/	0	-21.137	-5.752	0.465	21.91
Max Y	14463	121:SHORT-CF	0.500	-10.418	-2.298	12.866	16.71
Min Y	3870	239:SHORT-CF	0	13.807	-8.040	12.805	20.47
Max Z	13259	73:SHORT-CR/	0.266	4.825	-5.299	13.305	15.11
Min Z	9314	171:SHORT-CF	0.266	1.042	-5.677	-13.787	14.94
Max Rst	3870	225:SHORT-CF	0	23.267	-6.220	-0.242	24.08

## 9.6. slab-700mm

## Limit state of collapse

			Shear	(Local)	Men	Membrane (Local)			Bending Moment (Local)		
	Plate	L/C	SQX	SQY	SX	SY	SXY	MX	MY	MXY	
			(N/mm <sup>2</sup> )	(N/mm <sup>2</sup> )	(N/m m <sup>2</sup> )	(N/mm <sup>2</sup> )	(N/mm²)	(kN'm/m)	(kN'm/m)	(kN <sup>-</sup> m/m	
Max Qx	4014	3423:REVERS	0.851	-0.201	-0.085	-0.082	0.204	-365.879	-92.407	-20.46	
Min Qx	4119	2043:NORMAL	-0.874	-0.135	-0.006	-0.015	-0.078	-48.706	97.503	9.99	
Max Qy	3030	3706:SEISMIC	-0.704	0.753	-0.121	-0.022	-0.071	-190.651	-64.172	114.32	
Min Qy	4205	2103:NORMAL	-0.185	-1.102	-0.023	-0.016	0.040	-52.070	240.822	61.00	
Max Sx	4282	3657:REVERS	0.248	-0.558	0.754	0.094	-0.755	10.819	79.941	-95.17	
Min Sx	4282	3460:REVERS	-0.115	-0.324	-0.672	-0.093	0.691	-53.063	34.737	86.41	
Max Sy	3849	2133:NORMAL	-0.122	0.231	0.640	0.176	-0.226	-19.268	32.365	21.61	
Min Sy	4521	2026:NORMAL	-0.032	-0.582	-0.486	-0.225	-0.308	-48.852	-19.861	2.44	
Max Sxy	4282	3460:REVERS	-0.115	-0.324	-0.672	-0.093	0.691	-53.063	34.737	86.41	
Min Sxy	4282	3657:REVERS	0.248	-0.558	0.754	0.094	-0.755	10.819	79.941	-95.17	
Max Mx	4014	3694:REVERS	-0.549	0.091	0.039	0.065	-0.187	286.827	122.200	-7.85	
Min Mx	3018	3585:REVERS	-0.861	-0.224	0.081104	-0.050	-0.178	-373.532	-86.475	19.19	
Max My	4205	3261:REVERS	-0.357	-0.834	0.101	0.141	0.098	56.642	379.016	89.02	
Min My	4249	3081:REVERS	0.079295	-0.192	0.015	-0.080	0.029	-51.455	-291.359	8.25	
Max Mxy	4205	3477:REVERS	-0.488	-0.816	0.094	0.076	0.224	147.169	217.088	180.05	
Min Mxy	4595	3657:REVERS	0.531	-0.721	0.129	0.079	-0.236	190.319	197.733	-168.95	

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## Limit state of Serviceability

			Shear (Local) Membrane (Local)			cal)	Bending Moment (Local)			
	Plate	L/C	SQX	SQY	SX	SY	SXY	MX	MY	MXY
			(N/mm <sup>2</sup> )	(N/mm <sup>2</sup> )	(N/m m <sup>2</sup> )	(N/mm²)	(N/mm²)	(kN m/m)	(kN m/m)	(kN m/m
Max Qx	3267	165:SHORT-CF	0.465	0.059	0.022	0.001	0.027	-79.956	55.761	-1.44
Min Qx	4119	99:SHORT-CR/	-0.637	-0.104	-0.007	-0.010	-0.056	-34.498	68.772	6.18
Max Qy	2934	135:SHORT-CF	-0.570	0.440	0.009	0.006	0.025	-33.667	69.776	28.93
Min Qy	4205	159:SHORT-CF	-0.136	-0.750	-0.021	-0.008	0.022	-29.962	175.055	41.65
Max Sx	4521	69:SHORT-CR/	-0.163	-0.582	0.482	0.061	0.398	7.035	84.945	33.50
Min Sx	4521	100:SHORT-CF	-0.036	-0.419	-0.396	-0.135	-0.303	-25.176	56.513	-11.31
Max Sy	3849	189:SHORT-CF	-0.093	0.162	0.442	0.121	-0.153	-10.160	23.415	12.51
Min Sy	4521	82:SHORT-CR/	-0.010	-0.378	-0.325	-0.154	-0.205	-35.907	-28.463	0.24
Max Sxy	4521	189:SHORT-CF	-0.113	-0.547	0.473	0.092	0.415	-8.785	20.576	28.1
Min Sxy	4521	124:SHORT-CF	-0.077	-0.466	-0.373	-0.020	-0.357	-11.021	91.467	-6.92
Max Mx	3269	219:SHORT-CF	-0.179	-0.204	0.022	-0.000	0.001	102.738	130.512	-7.28
Min Mx	3007	213:SHORT-CF	0.358	-0.114	0.022	0.010	0.025	-125.318	-5.817	-9.68
Max My	4205	135:SHORT-CF	-0.142	-0.737	0.043	0.026	0.049	-25.874	187.524	41.33
Min My	4734	201:SHORT-CF	0.019	-0.260	-0.001	-0.020	0.000	-23.712	-180.080	5.89
Max Mxy	4205	165:SHORT-CF	-0.159	-0.732	-0.024	-0.019	0.047	-8.783	116.237	65.02
Min Mxy	4595	99:SHORT-CR/	0.231	-0.650	-0.160	-0.025	0.055	45.431	120.060	-59.22

## 9.7. slab-800mm

## Limit state of collapse

			Shear	(Local)	Men	nbrane (Lo	cal)	Bending Moment (Local)			
	Plate	L/C	SQX	SQY	SX	SY	SXY	MX	MY	MXY	
			(N/mm <sup>2</sup> )	(N/mm <sup>2</sup> )	(N/m m <sup>2</sup> )	(N/mm²)	(N/mm <sup>2</sup> )	(kN m/m)	(kN m/m)	(kN m/m	
Max Qx	11748	2010:NORMAL	0.884	0.646	-0.012	-0.010	0.005862	36.523	-101.622	75.80	
Min Qx	14373	2058:NORMAL	-0.911	0.604	-0.002	0.015	-0.000	0.915	-44.440	-70.32	
Max Qy	11748	2010:NORMAL	0.884	0.646	-0.012	-0.010	0.005862	36.523	-101.622	75.80	
Min Qy	9727	2172:NORMAL	-0.853	-0.652	-0.007	-0.015	-0.003	-30.681	-97.353	70.84	
Max Sx	11146	2163:NORMAL	0.039	-0.101	0.090	0.072278	-0.087	-25.422	5.720	2.04	
Min Sx	11147	2088:NORMAL	0.111	-0.164	-0.120	-0.137	0.141	-67.200	-69.447	34.83	
Max Sy	11146	2151:NORMAL	0.056	-0.060	0.047	0.082	-0.024	-46.369	76.413	5.81	
Min Sy	10170	2052:NORMAL	-0.121	-0.376	-0.074	-0.203	-0.072	-114.475	-117.426	-49.59	
Max Sxy	11147	2088:NORMAL	0.111	-0.164	-0.120	-0.137	0.141	-67.200	-69.447	34.83	
Min Sxy	10169	2070:NORMAL	-0.107	-0.151	-0.113	-0.142	-0.142	-67.769	-23.091	-23.42	
Max Mx	11 59 8	2172:NORMAL	0.011	-0.009	-0.003	-0.009	-0.006	225.286	246.718	-9.42	
Min Mx	14575	2110:NORMAL	-0.369	-0.041	0.001	0.026	-0.000	-210.645	85.749	-9.86	
Max My	12079	2009:NORMAL	-0.019	-0.009	0.005	0.008	0.008	131.620	297.206	-2.49	
Min My	14624	2081:NORMAL	-0.045	0.242	0.000	0.008	-0.001	-49.529	-241.218	-21.89	
Max Mxy	11 94 1	2082:NORMAL	-0.503	-0.483	-0.015	-0.024	-0.022	62.522	47.128	118.852	
Min Mxy	7764	2055:NORMAL	0.550	-0.518	-0.003	-0.016	0.010	-29.559	-33.720	-118.17°	

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## Limit state of Serviceability

			Shear (Local)		Mer	nbrane (Lo	cal)	Bendin	g Moment	(Local)
	Plate	L/C	SQX	SQY	SX	SY	SXY	MX	MY	MXY
			(N/mm <sup>2</sup> )	(N/mm <sup>2</sup> )	(N/m m <sup>2</sup> )	(N/mm²)	(N/mm <sup>2</sup> )	(kN m/m)	(kN m/m)	(kN m/
Max Qx	11748	66:SHORT-CR/	0.612	0.441	-0.008	-0.004	0.004	25.507	-73.245	49.4
Min Qx	14373	114:SHORT-CF	-0.635	0.421	-0.001	0.013598	-0.001	2.878	-31.924	-45.5
Max Qy	11748	66:SHORT-CR/	0.612	0.441	-0.008	-0.004	0.004	25.507	-73.245	49.4
Min Qy	11712	228:SHORT-CF	0.606	-0.442	-0.004	-0.010	-0.002	24.136	-66.052	-51.4
Max Sx	11146	219:SHORT-CF	0.023	-0.063	0.067	0.050	-0.070	-11.846	8.861	-0.5
Min Sx	11146	144:SHORT-CF	0.096	-0.247	-0.085	-0.121	0.102	-94.266	-53.488	45.5
Max Sy	14448	241:SHORT-CF	0.001	-0.059	-0.000	0.061	0.000	-0.755	11.282	6.8
Min Sy	10170	108:SHORT-CF	-0.082	-0.261	-0.047	-0.138	-0.049	-80.437	-94.871	-34.4
Max Sxy	11135	144:SHORT-CF	0.098	-0.086	-0.066	-0.092	0.105	-47.715	-9.247	25.2
Min Sxy	10157	126:SHORT-CF	-0.094	-0.077	-0.061	-0.093	-0.106	-46.812	21.519	-19.3
Max Mx	11 59 8	228:SHORT-CF	0.003	-0.007	-0.002	-0.008	-0.004	153.949	168.655	-6.3
Min Mx	14575	166:SHORT-CF	-0.267	-0.029	0.001	0.020	0.000	-152.733	61.013	-7.5
Max My	12079	65:SHORT-CR/	-0.013	-0.003	0.004	0.008	0.005	88.374	209.147	-1.6
Min My	14624	137:SHORT-CF	-0.032	0.173	0.000	0.007	-0.001	-35.391	-173.871	-15.7
Max Mxy	11941	138:SHORT-CF	-0.337	-0.331	-0.009	-0.015	-0.015	48.964	38.932	83.0
Min Mxy	11 49 7	228:SHORT-CF	-0.406	0.298	0.000	-0.007	-0.004	57.054	38.799	-80.4

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#### 10.0 Results and Discussions

The P- $\delta$  structural analysis was carried out to determine the structural responses such as member forces and deflections. The member loads will be used to design the RCC elements for ship repair facilities. The reinforcement results for each structural element are summarized in this section.

#### **10.1 Piles**

#### 10.1.1 Pile-1300mm

The design of pile is done for the moment adopted from the analysis done by STAAD Pro. The reinforcement provided is summarized in **Table. 10.1**.

Table. 10.1: Reinforcement summary for 1300mm

Dia (mm)	No of bars provided	% of reinforcement provided	Lateral ties provided	crack width (mm) Short term
1300	2x33T32	4.0%	12mm# @150mm spacing c/c	0.159

#### 10.1.2 Pile-1200mm

The design of pile is done for the moment adopted from the analysis done by STAAD Pro. The reinforcement provided is summarized in **Table. 10.2**.

Table. 10.2: Reinforcement summary for 1200mm

Dia (mm)	No of bars provided	% of reinforcement provided	Lateral ties provided	crack width (mm) Short term
1200	2x25T32	3.56%	12mm# @150mm spacing c/c	0.136

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#### 10.3 Column

The design of column is done for the moment adopted from the analysis done by STAAD Pro. The reinforcement provided is summarized in **Table. 10.3**.

Table. 10.3: Reinforcement summary for column (1750x1750mm)

Size (mm)	No of bars provided	% of reinforcement provided	Lateral ties provided	crack width (mm) Short term
1750x1750	1x34T32 on each side	3.5%	12mm# @150mm spacing c/c	0.18

#### **10.4 Beams**

The design of beam is done for the moment adopted from the analysis done by STAAD Pro. The reinforcement provided is summarized in **Table. 10.4**.

Table. 10.4: Reinforcement summary for beams

C		Doinfor	aamant	% of	f	Ctimoung	Crack widt	th (mm)
S. No.	Size(mm)	e(mm)   Reinforcement		reinforcement		Stirrups (mm)	Top	Bottom
110.		Bottom	Top	Bottom	Top	(11111)	Short	Short
1	1750x2000	30T32	30T32	0.73	0.73	12T 10 legged @125 c/c	0.061	0.038
2	1500x2000	30T32	30T32	0.85	0.85	12T 10 legged @100 c/c	0.101	0.091

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## **10.4 Slab**

The design of slab is done for the moment adopted from the analysis done by STAAD Pro.The reinforcement provided is summarized in **Table. 10.5**.

Table. 10.5: Reinforcement summary for slab

S.	Description	Reinforcement Rescription		Reinforcement % of		% of reinforcement		Stirrups		k width nm)
No.	Description					(mm)	Top	Bottom		
		Bottom	Top	Bottom	Top		Short	Short		
1	Slab(700mm thick)	25@150c/c	25@150c/c	0.51	0.51	12T 6 legged @150 c/c	0.051	0.047		
2	Slab(800mm thick)	25@100c/c	25@100c/c	0.44	0.44	12T 6 legged @100 c/c	0.035	0.016		

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#### 11.0 Summary

The proposed ship repair structure is found to be safe against the above mentioned forces. The capital cost estimates have been prepared for the proposed structure in phase wise development. The total cost of the project based on requirements, initial discussions and observation as per 2022 rates is about Rs. 141.84 Crores for Phase-I and Rs. 266.40 Crores for Phase-II. The total cost of project is Rs. 404.60 Crores. The bill of quantity is attached in Annexure-II.

[Prof. S.A. Sannasiraj]